

I-81 Corridor-Wide Design Guide

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Document Review and Approval

1) Design Guide Reviews

The following team supporting the I-81 Corridor Improvement Program from VDOT provided input into and reviewed this I-81 Corridor-wide Design Guide.

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VDOT therefore authorizes publication and use of this document by VDOT and Consultants / Contractors for projects in the I-81 Improvement Program Corridor.

2) Design Guide Revisions

The following notes revisions to this Design Guide:

Revision Number	Revision Date	Revision Information
0	11/04/2021	Initial Release
1	06/08/2022	Revised Section 3.3 and Added New Appendix F & G
2	07/26/2022	Revised Appendix G
3	11/03/2022	Added New Appendix H
4	02/01/2023	Revised Appendix G, Section "Process"



I-81 Corridor-Wide Design Guide







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Section 1: Guide Purpose and Goals

1) Introduction

The I-81 Corridor Improvement Program consists of innovative, targeted improvements that will have a substantial effect on the safety and reliability of a critical portion of our nation's infrastructure.

As a critical north-south backbone of the East Coast's freight network, the I-81 corridor is vital to the efficient movement of goods through Virginia. More than one-third of all trucks and nearly 50% of the state's value of goods are transported along the 325-mile corridor. I-81 has the highest per capita truck volume in Virginia.

Within Virginia, I-81 connects 30 colleges and universities, 21 cities and towns and 13 counties, and parallels the Blue Ridge Parkway making this program critical to supporting job growth and economic vitality while reducing congestion, enhancing safety and reliability, and improving quality of life for everyone in our region.

The 325-mile long corridor spans across three VDOT districts: Bristol, Salem, and Staunton. There are currently 64 I-81 Corridor Improvement Plan Projects that will consist of both operational and capital improvements. Some of these projects include additional traffic cameras to detect incidents, changeable message signs, speed change lane extensions, curve improvements, as well as adding or widening lanes in some sections of the corridor.

2) Guide Purpose and Goals

The purpose of this document is to provide guidance to assist project teams in decision making for the I-81 Corridor Improvement Plan project elements. This is intended as a guide only and not a mandate of how projects are to be developed or how decisions are to be made.

This guide has three main goals:

- a) <u>Decision Making Guidance</u> Clarify process that the project teams delivering projects in the CIP can use to evaluate and document a variety of project elements that are likely to be encountered on a number of projects in the CIP.
- b) <u>Corridor Consistency</u> Establish templates and guidelines for documentation that will provide consistency amongst the many projects in the CIP so that similar project issues are being evaluated and documented in similar ways.
- c) <u>Practical Design</u> It is expected that projects delivered through the CIP will be based on a design approach built upon Performance Based Practical Design Principles (PBPD, see IIM-LD-255) and Common Sense Engineering (CSE, see IIM-LD-235). CIP projects should seek to achieve a high return for minimal cost; while seeking to address the purpose and need in a cost-efficient way. Cost effective solutions based on Common Sense Engineering and Performance Based Practical Design lead to good use of program funding across many projects and good stewardship of program funding.

Section 2: I-81 Corridor Project Scoping

1) Items for Consideration

Project scoping is a critical element of project development as it sets the stage (and in many cases sets limitations) for how a project will proceed to final design, construction, and opening day. Scoping will put brackets around several items including budget, milestone schedule, project termini, limits of NEPA analysis, along with project purpose and need. It is suggested that the following items be considered in addition to traditional scoping items, when developing scoping documents for projects within the I-81 Improvement Program:

- Review the I-81 Corridor Improvement Plan
- Evaluate number of crashes and pinpoint problem areas
 - Review known substandard feature locations, such as
 - Shoulder
 - Superelevation
 - Bridge clearance
 - Low friction numbers
 - Review other factors, such as
 - Animal crossing strikes
 - Oncoming glare
 - Vegetation
- Identify reasonable limits of disturbance for NEPA evaluation.
- Don't forget Stormwater Management, constructability considerations in constrained Rightof-way or environmental areas, maintenance access.
 - Consider all potential stormwater management locations during the early stages
 - Consider outfall channels there may be a need for improvements and/or restoration
- Evaluate adjacent project limits and/or potential need for extending ramps and accel/decel lanes
- Review need for replacement of large cost existing assets (i.e., bridges, overhead signing) to maximize opportunities to impact highway section only one time.
 - Assess existing and proposed projects (State of Good Repair, Smart Scale, Highway Safety Improvement Program, etc.)
 - \circ Look for opportunities to combine projects with schedule and/or funding
 - Consult with District Structure & Bridge Office
- Evaluate utility relocation requirements, including impacts to schedule and cost
- Evaluate schedule impacts related to coordination with outside entities, such as railroads
- Identify existing substandard features
- Identify whether the project will require Limited Access change
- Determine whether the project requires FHWA oversight (i.e., Projects of Division Interest (PODI)
- Evaluate the level of traffic analysis required
- Allow sufficient time in schedule for Right of Entry notifications for any work outside of right of way

- Consider potential improvements or betterments to crossing routes (i.e., Adding width to typical sections, vertical clearance issues, future improvements)
- Lighting needs and desires
- Landscaping

2) Risk Management

Risk management is an important function of scoping as it becomes the foundation of determining how and what issues designers can address as project development and construction progresses. Project Management Office (PMO) processes must be followed to identify project specific risks, consider how similar risks have been addressed in the overall corridor, and consider whether this project risk is unique and requires a different (performance based) solution. Project teams must recognize that as a project in the I-81 Improvement Program, certain decisions may have far reaching impacts beyond an individual project. Precedents and expectations can be set for the remainder of the program or certain choices may be inconsistent with previous projects or decisions. Individual project teams should identify and raise risk elements to I-81 team management before finalizing scoping as it can become very difficult to make changes post scoping.

3) Modifying Project Elements

As the project team works to close scoping and develop the project, there are items that may be identified requiring a change to the planned I-81 CIP project elements. If a project scope, schedule, or budget is being considered for modification from the original I-81 CIP, project teams must consider if modified project elements are within the goals of the overall I-81 CIP and within the specific project purpose and need.

Once a change is identified, the I-81 Program Manager shall be consulted to ensure the need is within the goals of the overall I-81 CIP before proceeding. Additionally, the I-81 Program Manager will advise what level of documentation is required to approve a particular change. Within this guidance document, there are two classifications of changes: (a) Modifying Original Planned I-81 CIP Project Elements Before Closing Scoping and (b) Modifying I-81 CIP Project Elements after Scoping. The following sections describe each of these situations.

a) Modifying I-81 CIP Project Elements Before Closing Scoping

Before scoping (Activity 22) has been closed in the project schedule, project scope and budget require special consideration and potentially documentation prior to making changes. If the project schedule requires change, the standard review process applies. To confirm and document that an originally planned project element of scope or budget should be modified, the following process is outlined for decision making and documentation:

Scope changes

- Identify the potential for a scope change.
 - Examples might include:
 - Project limits
 - Additional elements
- Address how the project change addresses purpose and need.



- Example items might include:
 - More detailed analysis of crash data uncovers an item that changes the project intent
 - Traffic and operational analysis at PFI stage identifies additional capacity needs
 - Changes in bridge or other asset condition
- Document the change. (See Section 4 for decision making template).
 - This supports District, Central Office and Program Manager review and approval, as applicable.
 - o Provides justification of budget and schedule changes if needed.

Budget changes

- If additional funds above the limits listed below are requested, documentation will be required:
 - Total Project Budget < \$5,000,000
 - 20% or greater increase in funding requested
 - \$5,000,000
 Total Project Budget < \$10,000,000
 - \$1,000,000 or greater increase in funding requested
 - Total Project Budget > \$10,000,000
 - 10% or greater increase in funding requested or above \$5,000,000 increase in funding
- Identify the reasons why the budget requires change
 - Examples might include:
 - Project limit changes
 - Additional elements
 - Unit cost increases
- Document the change.
 - This supports District, Central Office and Program Manager review and approval, as applicable.
 - Provides justification of budget and schedule changes if needed.

Approval level will be based upon extent of change required (scope or value/percentage of change in budget) and the following individuals could be involved:

- VDOT Project Manager
- District Preliminary Engineer Manager
- I-81 Program Manager
- Central Office
- CTB

If the modification cannot be fully documented, justified, or funded, it may not be approved for implementation within the program.

b) Modifying I-81 CIP Project Elements After Scoping is Closed

After scoping (Activity 22) has been closed in the project schedule, project scope, schedule and budget all require special consideration prior to making changes. To confirm and document that an originally planned project element of scope, schedule or budget should be modified, the following process is outlined for decision making and documentation:

Scope Change (See above)

Schedule change

- Identify the changes to the project schedule
 - Specific schedule milestones changes with focus on:
 - Approve Willingness or Adopt Location/Design (47,49)
 - Authorize RW & UT Funds (52)
 - Utility Relocation by Others (67U)
 - Acquire Right-of-Way (69)
 - Obtain Environmental Permits (70)
 - Advertise Project (80)
 - Award Contract (84)
 - Administer Contract (91)
 - Early and late starts and finishes for the activities
 - Identify the reasons why the schedule requires change
 - Examples might include:
 - Project limit changes
 - Additional elements
 - Environmental requirements
 - Discuss any mitigations to recover schedule
- Document the change.
 - This supports District, Central Office and Program Manager review and approval, as applicable.
 - Provides justification of budget and schedule changes if needed.

Budget Change (See above)

Approval level will be based upon extent of change required (scope or value/percentage of change in budget and/or schedule) and the following individuals could be involved:

- VDOT Project Manager
- District Preliminary Engineer Manager
- I-81 Program Manager
- Central Office
- CTB

If the modification cannot be fully documented, justified, or funded, it may not be approved for implementation within the program.

4) Value Engineering:

Value Engineering (VE) may be required on an I-81 Program Improvement project depending on the value of the project. Recommended changes to the design at this stage should be evaluated for the same elements noted above to confirm applicability to a given project scope. Projects must follow the current VDOT guidance for formal VE based upon project value. Project Managers and Team Members must also review I-81 funding rules, program precedents and other program specific considerations before recommending any potential VE changes, to confirm intent does not deviate from these guideline and requirements.

5) Addressing Locality and Authority Requests

Many of the proposed improvements along the I-81 corridor will have direct or in-direct connections to local roads or other local infrastructure. The following items may need to be considered related to potential requests by localities to included improvements in an I-81 project.

- Is the proposed improvement within the purpose and need of the given I-81 project?
 - \circ $\,$ If no, then the item would be considered a betterment.
- Is the requested item already covered within another project or planned funding source?
 If yes, then the item would be considered a betterment.
- Is this a maintenance item that the locality should be covering with local funding?
 If yes, then the item would be considered a betterment.
- What is the timeline for implementation of the item, and can the item be reasonably be anticipated to occur and/or be required within the I-81 design year horizon?
 - o If no, then the item would be considered a betterment.
- Is the proposed improvement part of an approved plan (comprehensive plan, bike and pedestrian plan, etc.)?
 - o If no, then the item would be considered a betterment.

If an item is identified as a betterment, the cost of this item will not be included in VDOT project funding associated with the I-81 CIP Projects. If a locality or authority wants to add funds to a project to fully cover the cost of inclusion of the betterment, this should be coordinated with the VDOT Program Manager and VDOT Project Manager. The locality or authority shall make the request prior to closing the project scope and the funds shall be committed from the locality prior to acquiring design approval.

Section 3: I-81 Projects Development

1) Practical Design Application - Corridor Considerations

VDOT has implemented the Location and Design Division Instructional and Informational Memorandum (I&IM) 255 - Performance Based Practical Design (PBPD) and I&IM 235 - Common Sense Engineering (CSE) and Context Sensitive Solutions to Transportation Challenges for use in developing VDOT projects. Per IIM-LD-255 and IIM-LD-, PBPD is a "design approach in which key design decisions are made with consideration of their anticipated effects on aspects of future performance that are relevant to the project purpose and need" and "the goal of CSE is to produce effective projects that optimize system wide transportation improvements through the use of appropriate design flexibility that provides for essential improvements", respectively. VDOT aims to utilize Practical Design and Common Sense Engineering approaches across the spectrum of projects planned with the I-81 Program Improvements. There are several items noted below for VDOT and Design Team consideration as Practical Design Applications are considered in the corridor:

- Maintain the existing or improve the safety level of individual projects and specific project elements
- Evaluate adjacent project planned improvements and consider any impacts to the corridor
- Assess performance based needs for upgrading existing elements if not impacted by planned improvements while considering the short term and long-term safety, operational and/or maintenance benefit before proposing upgrades
- Evaluate performance based needs for upgrading existing elements impacted by the project when such element is not a planned I-81 CIP project element.
- Consider other cost and schedule effective solutions that may achieve the project scope

2) Lessons Learned / Program Guidance

VDOT's Bristol District, Salem District, Staunton District, and Central Office have been developing a variety of projects along the I-81 corridor as part of the Improvement Program. As this program has progressed, there are several items that have been identified as lessons learned. These items are listed below and recommended for consideration during project development:

Schedule

- With the increase in interstate corridor projects, Limited Access change requests are also increasing. Schedules need to include adequate time for CTB approvals after Public Involvement and before Design Approval. Early and frequent coordination with the Central Office is necessary to stay on schedule. Refer to the <u>VDOT Road Design Manual</u> (Chapter 2E) for more information regarding Limited Access changes requirements.
- Project development schedules and advertisement dates should consider adjacent project schedules and potential overlap of work zones/contractors. Coordination between adjacent projects should occur (i.e., staggering schedules) to minimize/avoid negative impacts to either project or to create synergies (i.e., combined work zones) between the projects.

- Include time in the schedule for Programmatic Categorical Exclusions (PCE) and one-time PCEs (OTPCEs) and for preliminary engineering work (e.g., cores/borings, surveys, etc.). Consult with District Environmental for additional guidance for specific activities.
- Include time for NPDES General Construction Permit for land disturbance activities during PE phase (e.g., geotechnical investigations.).

Shoulders

- Shoulder DE/DW flexibility is recommended on bridges to avoid unnecessary structural widening expenses that aren't required to meet project intent.
- The program includes many widening projects, and some require permanent concrete barriers adjacent to reduced shoulders, due to project constraints. Drainage spread adjacent to the barrier must be considered in determining the appropriate minimum shoulder width along the barrier.
- Consider the existing shoulder widths and corridor consistency in decisions of proposed shoulder widths.

Ditch Width / Cut Slope

• Evaluate shoulder widths, ditch widths, and/or cut slope requirements to ensure that slopes aren't cleared and reworked where there is little to no benefit in doing so.

Removal of Trees and Vegetation

- Removal of trees and/or vegetation lead to perceptions of noise increases by nearby residents despite noise modeling showing vegetation doesn't dampen noise.
- Consider native tree and shrubs be replanted.

Vertical Curves

• With the increase in design and posted speeds on interstates, some existing vertical curves may not meet stopping sight distance standards. These locations only require reconstruction and correction if there is evidence of associated accident clusters or the feature is identified as a specific part of the project performance needs.

Cross Slope/Superelevation

 While AASHTO policy states that widening should not be designed in a way that slows drainage flow across the cross section (a steeper cross slope flowing to a flatter cross slope), the design team will need to carefully study and evaluate the actual implications and interaction of longitudinal grade on drainage. Consideration should be given to potential constraints that may not allow for correcting such conditions due to compounding effects that may require extensive reconstruction beyond the purpose and need of a given project. It is recommended that the need for or justification for not correcting this condition be clearly documented in the scoping documentation.

- A rolling average cross slope (computed over segment lengths of 500ft) is recommended when analyzing long tangent sections for compliance to normal crown cross slopes. This rolling average would then be utilized to determine whether cross slope correction may be required.
- Existing cross slopes in tangent sections that are consistently flatter than 0.5% for lengths greater than 500 linear feet should be considered for correction.
- Widening may extend the existing cross slope/superelevation or be constructed on a corrected cross slope/superelevation, depending on the specific situation. A combination may be needed to avoid drainage ponding concerns.
- Widening of an existing cross slope/superelevation that slopes opposite of the curve direction is discouraged.
- Cross slope or superelevation correction may not be a beneficial improvement if it results in a substandard bridge clearance.
- The program includes many widening projects that create three lane sections (in one direction) with paved shoulders on both sides of the lanes. Normal crown typical sections that slope all lanes in one direction are typically not desired due to ice/snow concerns from drainage flowing across multiple lanes (and shoulders). However, consideration should be given to constraints that may not allow for such corrections to the typical section due to extensive reconstruction beyond the purpose and need of a given project. It is recommended that the need for or justification for not correcting this condition be clearly documented in the scoping documentation.

Scope/Funding

- I-81 program funding is intended to improve the safety and operations of the interstate corridor. Scope creep to improve connecting facilities or accommodate new facilities that are not funded should not be allowed unless they contribute directly to interstate performance.
- While the I-81 study identified approved projects, the appropriate traffic operational and safety analyses are still required as documentation per TOSAM. It is important that any analyses/reports focus on alternatives that meet the project purpose and need identified rather than encouraging scope creep and improvements beyond the interstate performance.
- I-81 program funding is not intended for maintenance use. Other maintenance funding sources must be used to cover additional maintenance scope features that are beyond the project purpose and need.
- Project limits were determined from a corridor study and will likely require refinement. This refinement must be justified by the project safety and operational analysis and must be consistent with the approved project purpose and need.

FHWA Involvement

• FHWA currently designated major widening projects and Design Build projects along the I-81 corridor as Projects of Divisional Interest (PODIs). For these projects requiring Federal oversight, ensure the FHWA representative is included in major milestones, plan submissions, traffic analysis and major decisions. The PODI determination is subject to change.

Preliminary Engineering Investigations

- Identify stormwater/E&S and environmental clearances needed for preliminary engineering investigations
- OTPCEs for work within the right of way
- Consider bat surveys which may be helpful/necessary prior to tree cutting for preliminary engineering investigations, such as cutting trees for soil borings and bridge inspection and testing
- PCEs for investigations outside of the right of way
- PCEs for other activities within the right of way not covered under an OTPCE
- E&S Plan and Inspections
- General Construction Permit (NPDES) Coverage for land disturbance activities
 - E&S Plan, SWPPP, Posting of Documents, Inspections.

3) Disapproved Items

Several design and construction elements have previously been considered within the I-81 Program and a determination has been made that they are not approved for use in this corridor. Items that will not make it through an approval process are typically vetted during scoping and design progression. Therefore, individual disapproved items will not be listed here, but it is noted that elements which have typically not moved forward in design progress are due to the following considerations:

- Beyond project scope of the I-81 Program and/or funding constraints
- Operational concerns
- Constructability concerns
- Maintenance concerns (What about repairing an existing storm drain pipe? Is this included or excluded.
- Inconsistency with existing or planned corridor features
- Could preclude a future planned improvement
- Inconsistency with local comprehensive plan
- Lack of substantial safety or operational performance benefit

If a Design Team feels that due to the specific circumstances of an individual project that a previously disapproved item should be considered and approved for the project, the PM should follow the guidance in <u>Section 4: Decision Documentation Templates</u> to request approval and deviation from a specific item. This approach should be utilized for identified items within this manual or other I-81 Program guidance.





Section 4: Decision Documentation Templates

The following pages outline a potential template for use in documenting decisions for projects included in the I-81 Improvement Program. Regardless of format, any approved decision documentation should be uploaded into the ProjectWise project folder by the PM.

It is noted that the templates included in this section or in the appendices of this guide do not supersede the need for standard Design Exception, Design Waiver, or other standard documentation. These templates are provided as a means to document decisions made as part of the I-81 Corridor Improvement Program.



I-81 PROGRAM IMPROVEMENT PROJECT DECISION DOCUMENTATION

UPC: Project Name:

Project No.:

Issue – This should be 1-2 sentences in simple terms for project/District leadership to easily understand and comprehend quickly.

Decision – Document the decision and provide 1-2 sentences in simple terms for each of the following impacts:

- Decision Description
- Design (DE/DW or policy use/deviation):
- Constructability:
- Cost:
- Schedule:
- Environmental:
- Right-of-Way & Utilities:
- Maintenance:
- Operations:
- Safety:

Prepared by:

Name & Title

Date

Supporting Documentation Sections

Background – Provide technical background of the issue including any known problems and issues if left unresolved. This should include any relevant applicable design standards, examples of current issues, scale and scope of the issue, limiting elements, constructability concerns, environmental considerations, and consequences of no build.

Alternatives Analysis – Identify each alternative considered as a potential solution for this issue. Include any technical, funding, or other important details relevant to the decision process. Adverse outcomes of each alternative should be part of the discussion. This may require interdisciplinary coordination.

Decision & Justification – Provide discussion on selected alternative and why it was selected. This should include any relevant standard modifications, use of Context Sensitive Solutions, Design Waivers/Exceptions required, and other relevant decision points.

Prepared by – Identify the author of the documentation. This form would typically be prepared by the project team in consultation with the VDOT Program Manager and Central Office.





I-81 Corridor-wide Design Guide

Appendix A





Appendix A: DE/DW for Superelevation and Cross Slope

Information needs:

- Existing roadway plans showing typical sections and curve data with design superelevation
- Proposed roadway plans showing typical sections and curve data with design superelevation
- Aerial imagery of roadway
- Survey of pavement edge lines and existing lane striping
- Digital terrain model or survey elevations of existing pavement
- Crash history of past 3 years within project limits with crashes geospatially located

References:

- AASHTO Green Book Chapter 4.2.2.1 (Tangent Cross Slope)
- AASHTO Green Book Chapter 3.3.3.2 (Maximum Superelevation)

Process:

- <u>Curves</u>
 - Establish the pattern or trend of existing superelevation through all horizontal curves within the project limits
 - Determine both the proposed superelevation rate and required AASHTO minimum superelevation rate for each horizontal curve
 - Populate the Summary of Superelevation Rates spreadsheet as shown in Exhibit A-1 for inclusion as a table in the Design Exception Narrative
 - Calculate the average superelevation of the existing curve between the 1/3 Lr points (1/3 in and 1/3 out)
 - Identify horizontal curves with existing average superelevation rates that are more than 0.5% less than the current design superelevation rate standard (this analysis is for identification and documentation of need for DE/DW and is not for evaluating survey accuracy needs)
 - Conduct ball bank indicator study on curves that do not meet current design standards and document results
 - Establish the shoulder rollover on the opposite side of widening (i.e., if widening to the inside, verify the right shoulder). Flag locations where the rollover exceeds 7% between the existing cross slope of the adjacent lane and the existing shoulder cross slope.

<u>Tangents</u>

 Populate the Summary of Tangent Cross Slopes spreadsheet as shown in Exhibit A-2 for inclusion as a table in the Design Exception Narrative

A-1

IMPROVE 81

- Calculate the rolling average cross slope in tangent sections segmented into segment lengths of 500'
- Identify segments (500') where existing cross slope is less than 1.5% or greater than 2.5%
 - Flag segments where existing cross slope is less than 1.0% or greater than 3.0% (these areas may potentially require mitigation regardless of the results of the crash data analysis)
- If a crown exists, establish the rollover across the crown line. Flag locations where the rollover exceeds 4% between the existing cross slopes of the lanes on either side of the crown.

<u>Tangents and Curves</u>

- Establish the existing cross slope for each lane throughout the project corridor at a minimum of 50' intervals.
- Populate the Cross Slope / Superelevation Evaluation spreadsheet as shown in Exhibit A-3.
- Evaluate 3 years of crash data with specific focus to the areas of superelevation rate and/or tangent cross slope that do not meet current standards.
 - If crash data suggests an existing safety problem that aligns with locations of deficiencies and where the substandard superelevation/cross slope is likely a contributing factor to the crashes, evaluate appropriate mitigation measures and/or corrections
 - Issues to be considered consist of:
 - Impacts to side slopes if variable depth wedging or cross slope correction is applied
 - Constructability and maintenance of traffic
 - If crash data does not suggest an existing safety problem that aligns with locations of deficiencies or there is no discernable crash trend, consider mitigations offered by the proposed project such as widened shoulders, improved drainage infrastructure, rumble strips, etc.
- Fill out IIM Form LD-440 or LD-448 including supporting documentation.

Supporting Documentation:

Attachment A – Existing roadway plans (Title Sheet and Plan Sheets)

Attachment B – Excerpts from AASHTO and VDOT standards as applicable to the request

Attachment C – Proposed roadway plans (Title Sheet, Typical Sections, and Plan Sheets)* *Profiles and Sample Cross Sections may be included if needed

Attachment D – Cross Slope / Superelevation Evaluation spreadsheet (as shown in Exhibit A-3) (*The table format may need to be modified slightly to fit a project specific situation, based on number of lanes and location of widening. Table alignment to follow the proposed typical section in direction of stationing.*)

Attachment E – 3 years of crash data in tabular format and geo-spatial exhibits

Attachment F – Mitigation measures documentation (e.g., ball bank indicator results, etc.)



Attachment G – Miscellaneous Information

Notes:

• Should the calculated cross slopes and/or superelevation show widely varying cross slopes in a given section, the project team should consult with the District Survey Division to confirm if accuracy of aerial survey data is sufficient for design and/or documentation of the existing conditions.

A sample approved DE/DW is attached at the end of this section.

A-3





A-4

Figure 2: Decision Flowchart for DE/DW for Cross Slope



I-81 Corridor Improvement Plan

Project Name

Date:

Summary of Superelevation Rates

VDOT Project #:

VDOT UPC #:

Curve ID	PC Station	PI Station	PT Station	Length of Curve (ft)	Radius (ft)	Design Speed (mph)	AASHTO Min. Superelevation Rate ¹ (%)	Averag Fu Superel (%) Outside Lane (%)	e Exist. Ill evation 6) Inside Lane (%)	Approx. Actual Design Speed ² (mph)
CURVE 001	100+00.00	110+00.00	120+00.00	2,000	5,725	70	4.00%	0.50%	-2.70%	45

Exhibit A-1: Curve Summary

A-6



I-81 Corridor Improvement Plan

Project Name

Date:

Summary of Tangent Cross Slopes

VDOT Project #:

VDOT UPC #:

Tangent Stati	on F	Range	Length of Tangent	Design Speed	Crown?	Average Exist. Cross Slope (%)		
From		То	(ft)	(mph)	(YES/NO)	Outside Lane (%)	Inside Lane (%)	
100+00.00	to	105+00.00	500	70	YES	0.50%	1.00%	

Exhibit A-2: Tangent Summary

A-7

VDOT

I-81 Corridor Improvement Plan

Project Name

Cross Slope / Superelevation Evaluation

VDOT Project #:																
VDOT UPC #:																
														Date:		
	Southbound Lanes								Northbound Lanes							
Station		VDOT/AASHTO (2020) Standard ¹	Existing Lane (Ri Cross	g Outside ght Lane) 5 Slope		Existing I (Left La Sl	nside Lane ne) Cross ope	Proposed Widened Lane (New Left Lane)		Proposed Widened Lane (New Left Lane)	Existing I (Left La Sl	nside Lane ne) Cross ope		Existing Lane (Ri Cross	; Outside ght Lane) s Slope	
			Existing (%)	Required (%)		Existing (%)	Required (%)	Cross Slope (%)		Cross Slope (%)	Existing (%)	Required (%)		Existing (%)	Required (%)	
122+50			-1.00%	-1.00%	-1.00%	-2.60%	-1.50%	-1.50%		-1.80%	-1.80%	-1.80%		-2.00%	-2.00%	
123+00	uest	Transition	-1.40%	-1.40%		-0.40%	-1.80%	-1.80%		-1.40%	-1.40%	-1.40%		-1.00%	-1.00%	
123+50	n Req	Transition	-2.70%	-2.70%	5	0.60%	-1.40%	-1.40%		-2.70%	-2.70%	-2.70%		0.01%	0.01%	
PC 123+73.11	otio				FG								FG			
124+00	Excel	Transition	-2.20%	-2.20%	ne/PO	1.70%	-0.90%	-0.90%		-2.20%	-2.20%	-2.20%	ne/PO	1.00%	1.00%	
124+50	Jesign	4.0	-2.30%	-2.30%	Baseli	0.80%	2.70%	2.70%		-2.30%	-2.30%	-2.30%	Baseli	1.00%	1.00%	
125+00	its of I	4.0	-4.00%	-4.00%		0.40%	2.70%	2.70%		-4.00%	-4.00%	-4.00%		1.00%	1.00%	
125+50	Lim	4.0	-3.20%	-3.40%		-0.10%	2.70%	2.70%		-3.20%	-3.20%	-3.20%		1.00%	1.00%	
126+00		4.0	-3.40%	-3.40%	0% -30	-30.00%	2.70%	2.70%		-2.60%	-2.60%	-2.60%		1.00%	1.00%	

Exhibit A-3: Cross Slope and Super Evaluation Rev

A-8



VIRGINIA DEPARTMENT OF TRANSPORTATION LOCATION AND DESIGN/STRUCTURE & BRIDGE DESIGN EXCEPTION REQUEST

Design Exception Number:

(For use by NOVA Only)

Date: C	Date: Click to enter a date.										
То:	Vernon Heishn	nan, PE		Assistant State Location and Design Engineer							
From:	Tamara Pritcha	ard, PE		District Location and Design Engineer							
Subject	DESIGN EXCEP	TION REQUEST									
Project	Project Information										
UPC		116156	State Project Number		(FO) 0081-095-829						
Federal Project Number		NHPP-081-1(362)	District		Bristol						
City/Cou	nty	Washington	Click to choose an item.								
Start Loc	ation (From)	MM 32.197 (NB)									
End Loca	ition (To)	MM 33.519 (NB)									
Project D	Description	I-81 Northbound Truck Climbing Lane									
Funding	Source										

Desi	Design Exception Request For												
	Design Speed		Horizontal	Horizontal Curve Radius				Design Loading Structural Capacity*					
	Lane Width		Cross Slop	e				Vertical Cle	Vertical Clearance*				
	Shoulder Width		Sight Dista	nce (S [.]	toppin	ng)	\square	Supereleva	ition Ra	te			
	Maximum Grade		Other	Other									
* The	* These are typically requested by the Bridge designer.												
Road	Road and Traffic Information												
Curre	ent ADT	16,040			Design ADT				21,404				
% Tri	ucks	17		gn Speed		75	Posted Speed		70				
Redu Spee	iced Design d (if applicable)					Is Project on NHS?			Yes				
Func Class	tional sification	GS-INT INTERSTATE											
Min. Stan	AASHTO dard	4.0%				AASHTO Reference			2011 AASHTO Green Book Table 3-10b (Page 3-47)				

LD-440 (5-22-20)					Page 2 of 4				
Existing Dimensions	Varies 2.8% m max. exist. ou	in. to 4.0% tside lane	Requested Dimensions	Vari max	es 2.8% min. to 4.0% «. exist. outside lane				
Total estimated constru	ction cost of pr	oject \$ 14,5	\$ 14,500,000						
(Based on approval of th or replacing structures,	nis exception) acquiring addit	Cost should inc ional right of w	clude item such as additional g vay, wetland mitigation, etc.	gradin	ng or paving, widening				
Additional cost to meet minimum AASHTO standard \$ Please see attached narrative									
Background description	on of project:	(Attach Sepa	rate Document)						
(Include a description of the proposed exception nonstandard features w applicable plan sheets, p	f the general ch . Provide a brie hen relevant to profiles and cro	aracteristics of of description of the proposed ss sections.)	f the existing highway focusing of the adjacent highway segme exception as well as the title s	g on tl ents, h sheet,	he features relevant to nighlighting existing , typical section sheet,				
Design Exception Deta	ails								
Purpose and need for ex standards cannot be me any future plan to upgra exception to standard)	cception; why it (include ide this	Please see at	ase see attached narrative						
Are there any plans to ir	nprove the app	roach roadway	y within the next ten (10) year	s?	Please see attached narrative				
Accident history for the number of crashes, seve crashes, types of crashe the road, rear-ends, side head-on, etc.), cause of affect the design except on types and number of a comparison of the stat average	past 3 years, erity of s (run- off- eswipes, crashes, the ion will have crashes and tewide	Please see at	Please see attached narrative						
Effect of design exception	on on safety	Please see at	tached narrative						
Mitigation of the substa element(s)	ndard design	Please see at	tached narrative						
Has the Responsible Dis Engineer reviewed the s design and do they agre proposed mitigation me not, please explain.	trict Traffic ubstandard e with the asures? If	Please see At	e see Attachment E						

LD-440 (5-22-20)



Prepared By: Chase Buchanan, PE Date: 4/22/2021 District Roadway Design Manager

Note: The responsible person that prepares the request shall also electronically seal and digitally sign in the block above. **All signatures below shall be digital signatures**.

Remarks:

Recommended For Submission to C.O. By								
	District Location and Design Engineer							
	Or							
	District Structure and Bridge Engineer							

Recommended for Approval By										
		Assistant State Location and Design Engineer								
		Or								
	-	Assistant State Structure and Bridge Engineer								
	-									
		(For Crossovers Only)								
VDOT and FHWA A	pproval									
		State Location and Design Engineer								
VDOT Approval By		Or								
		State Structure and Bridge Engineer								
FHWA Approval By										

Cc: Project Manager Design Engineer State Geometric Design Engineer Design Exception – Superelevation Rate I-81 NB Truck Climbing lane UPC 116156

DESIGN EXCEPTION NARRATIVE

Background Description of Project:

The project is part of the I-81 Corridor Improvement Program that will provide operational and capital improvements along the 325-mile long corridor that spans across three Virginia Department of Transportation districts: Bristol, Salem, and Staunton.

I-81 is classified as an Interstate and is part of the National Highway System (NHS). The existing typical section through the project limits is a four-lane, divided, rural facility with two 12 ft. lanes, a 7 ft. inside shoulder with 3 ft. paved, and a 15 ft. outside shoulder with 10 ft. paved, in each direction. The northbound and southbound roadways are separated by a grass median that varies 25 ft. to 65 ft. in width. An existing double faced guardrail, predominately located adjacent to the northbound inside shoulder, runs along the median within the project limits. In some locations, standard guardrail is located adjacent to the median shoulders and the outside shoulders for the northbound and southbound directions.

Located north of I-81 Exit 32 between Mile Marker 32.197 (NB) and Mile Marker 33.519 (NB), the project will improve safety and operations by widening the existing northbound roadway to accommodate the addition a 12-foot truck climbing lane with a total outside shoulder width of 12 feet and a paved shoulder width of 10 feet. The truck climbing lane will begin at the end of the existing Exit 32 on-ramp acceleration lane and continue for approximately 4,800 feet. The project's purpose and need does not include any improvements to the existing mainline travel lanes. The existing mainline travel lanes will be resurfaced when construction is complete to obscure pavement markings for a lane shift during construction. The design speed for this section of I-81 is 75 mph (posted 70 mph).

The beginning and end of the project fall within a portion of two existing northbound horizontal curves. In accordance with the *VDOT Road Design Manual Geometric Standard GS-INT*, this section of I-81 should be designed with a maximum superelevation rate of 8%. The original project was classified as a Class I Divided Roadway in Rolling Terrain and designed in accordance with the 1953 Virginia Department of Highways (VDH) Road Design and Standards. All horizontal curves were designed in accordance with Standard TC-2. The curve data on the original plans shows an EB (NB) Centerline radius of 5,729.58 feet for the horizontal curve entering the proposed project limits and an EB (NB) Centerline radius of 5,729.58 feet for the horizontal curve exiting the proposed project limits. In accordance with the 1953 VDH Road Design and Standard TC-2, horizontal curves less than 2° (R > 2,864.79') shall be superelevated by an amount equal to the standard pavement crown of $\frac{1}{4}$ ":1'. A copy of the original Title Sheet, Plan Sheets, and 1953 Road Design Standard are presented in Attachment A.

In 2010, the General Assembly of Virginia passed HB 856 to allow the maximum speed limit to be raised from 65 mph to 70 mph on interstates and certain other highways. A copy of HB 856 is included in Attachment B.

DESIGN EXCEPTION DETAILS

Purpose and need for exception; why standards cannot be met (include any future plans to upgrade this exception to standard):

Superelevation is the amount by which the outer edge of curve on a roadway is banked above the inner edge to allow fast-moving vehicles to pass through a curved path without overturning or skidding. The maximum rate of superelevation is based on climate conditions, terrain conditions, type of area, and frequency of very slow moving vehicles whose operation might be affected by high superelevation rates.

There are two horizontal curves located within the project limits. Based on a review of existing survey data provided for the project, the horizontal curves were evaluated for compliance with American Association of State Highway and Transportation Officials (AASHTO) criteria for minimum superelevation rates. The results of the evaluation are presented in Table 1. Please refer to the plans provided in Attachment C for horizontal alignments and cross sections including stationing referred to in Table 1.

Curve No.	PC Station	PI Station	PT Station	Design Speed (mph)	Radius (ft)	AASHTO Min. Superelevation ¹ (%)	Existing Superelevation (%)
I-81NB_3	106+66.55	118+39.97	129+81.38	75	5,729.58	4.0	Varies 2.5
							to 4.0
I-81NB_6	162+34.08	175+05.48	187+36.34	75	5,729.58	4.0	Varies 2.3
							to 4.8

Table 1 – Summary of Superelevation Rates

1. AASHTO – A Policy on Geometric Design of Highway and Streets (2011), Section 3.3.5, Table 3-10b

As noted in Table 1, the two existing horizontal curves do not comply with the AASHTO minimum requirements for minimum superelevation rates. The existing conditions approximate a design speed of 65 mph for the I-81NB_3 outside lane and 70 mph for the I-81NB_6 outside lane.

The main objective of the project is to improve safety and operations by widening the existing roadway to accommodate a truck climbing lane. It is outside the project scope to bring the superelevation for each curve up to the current VDOT/AASHTO (2011) standard of 4.0%. Reconstruction of the roadway to achieve a compliant superelevation rate meeting the AASHTO criteria would require modifications to the existing mainline pavement cross slope(s) beyond the project limits and would result in additional grading and paving. It is anticipated that the additional grading and paving would increase the construction cost significantly as well as impacting the construction duration and schedule.

The purpose of this design exception is for the proposed truck climbing lane to match the existing outside lane cross slope which is less than the required 4.0% cross slope on the mainline. As reflected in Table 2 below, the proposed improvements will create a consistent and smooth lane change condition between the proposed truck climbing lane and existing outside lane. Table 2 shows the existing and proposed cross slope for each of the northbound travel lanes and the proposed northbound truck climbing lane. The VDOT/AASHTO (2011) standard would bring the travel lanes up to 4.0%.

Table 2									
I-81 Northbound Truck Climbing Lane									
		Ν	/lile	Mark 32.19	7 to Mile M	lark 33.519			
		Cr	oss :	Slope / Sup	erelevation	Evaluation'	k		
Station	Inside (L	eft) Lane		Outside (F	Right) Lane	Truck Clir	nbing Lane	VDOT/AASHTO	
110.00	Existing	Proposed		Existing	Proposed	Existing	Proposed	(2011) Standard ¹	
118+00	1.0	1.0		-2.9	-2.9	N/A	-2.9	-4.0	
118+50	2.2	2.2		-3.0	-3.0	N/A	-3.0	-4.0	
119+00	1.2	1.2		-3.2	-3.2	N/A	-3.2	-4.0	
119+50	0.5	0.5		-3.1	-3.1	N/A	-3.1	-4.0	
120+00	1.1	1.1		-3.0	-3.0	N/A	-3.0	-4.0	
120+30	1.1	1.1		-3.3	-3.3	N/A	-3.3	-4.0	ST
121+50	0.9	0.9		-4.0	-4.0	N/A	-4.0	-4.0	QUE
122+00	0.6	0.6		-3.9	-3.9	N/A	-3.9	-4.0	REC
122+50	0.2	0.2		-3.8	-3.8	N/A	-3.8	-4.0	NO
123+00	1.2	1.2		-3.2	-3.2	N/A	-3.2	-4.0	IL
123+50	0.8	0.8		-2.8	-2.8	N/A	-2.8	-4.0	XCE
124+00	-0.4	-0.4		-2.5	-2.8	N/A	-2.8	-4.0	Ш Z
124+50	0.0	0.0		-2.9	-2.9	N/A	-2.9	-4.0	SIG
125+00	1.5	1.5		-2.8	-2.8	N/A	-2.8	-4.0	DE
125+50	1.2	1.2		-2.5	-3.0	N/A	-3.0	-4.0	OF
126+00	0.2	0.2		-3.3	-3.3	N/A	-3.3	-4.0	ИТS
126+50	0.3	0.3		-2.4	-3.1	N/A	-3.1	-4.0	2 L
127+00	1.1	1.1		-3.0	-3.0	N/A	-3.0	-4.0	
127+50	1.0	1.0		-2.9	-2.9	N/A	-2.9	-4.0	
128+00	2.1	2.1		-2.8	-2.8	N/A	-2.8	-4.0	
128+50	0.8	0.8		-3.4	-3.4	N/A	-3.4	-4.0	
129+00	1.1	0.1		-3.9	-3.9	N/A	-3.9	-4.0 Transition	
PT 129+30	-0.1	-0.1		-4.5	-4.5	N/A	-4.5	Transition	
130+00	-0.7	-0.7		-3.7	-3.7	N/A	-3.7	Transition	
130+50	-1.3	-1.3	Ц В В	-3.2	-3.2	N/A	-3.2	Transition	3 AL
131+00	-1.4	-1.4	E/P	-2.5	-2.5	N/A	-2.5	Transition	NTI
131+50	-1.6	-1.6	E	-2.2	-2.2	N/A	-2.5	NC	L CE
132+00	-2.9	-2.9	ASE	-2.5	-2.5	N/A	-2.5	NC	ГОД (<u>а</u>
132+50	-2.0	-2.0	8	-2.4	-2.4	N/A	-2.5	NC	Y VI STE
133+00	-3.1	-3.1		-3.1	-3.1	N/A	-2.5	NC	D B QUE
133+50	-2.7	-2.7		-1.6	-1.6	N/A	-2.5	NC	REC
134+00	-3.0	-3.0		-2.1	-2.1	N/A	-2.5	NC	VO TO
134+50	-2.9	-2.9		-2.5	-2.5	N/A	-2.5	NC	S PF
135+00	-2.6	-2.6		-1.8	-1.8	N/A	-2.5	NC	TICE
135+50	-4.1	-4.1		-2.2	-2.2	N/A	-2.5	NC	RANCEF
136+00	-4.2	-4.2		-1.9	-1.9	N/A	-2.5	NC	N EX
136+50	-2.2	-2.2		-1.8	-1.8	N/A	-2.5	NC	N TO
137+00	-2.9	-2.9		-1.8	-1.8	N/A	-2.5	NC	DES
138+00	-2.0	-2.0		-1.9	-1.9	N/A	-2.5	NC	N O
138+50	-2.6	-2.6		-1.3	-1.3	N/A	-2.5	NC	3PE 202
139+00	-1.6	-1.6		-1.4	-1.4	N/A	-2.5	NC	/8/ SLC
139+50	-2.3	-2.3		-2.1	-2.1	N/A	-2.5	NC	SSC D 7
140+00	-2.1	-2.1		-2.3	-2.3	N/A	-2.5	NC	CR
140+50	-2.3	-2.3		-2.5	-2.5	N/A	-2.5	NC	SED IL D
141+00	-2.7	-2.7		-3.3	-3.3	N/A	-2.5	NC	PO
141+50	-1.9	-1.9		-2.7	-2.7	N/A	-2.5	NC	PRO A Ei
142+00	-2.0	-2.0	1	-3.0	-3.0	N/A	-2.5	NC	N F
142+50	-1.5	-1.5		-2.4	-2.4	N/A	-2.5	NC	TIC
143+00	-2.3	-2.3	1	-2.4	-2.4	N/A	-2.5	NC	SEC
143+50	-2.1	-2.1		-2.8	-2.8	N/A	-2.5	NC	INT
144+00	-2.3	-2.3	-	-3.5	-3.5	N/A	-2.5	NC	NGE
144+50	-1.9	-1.9	-	-2.4	-2.4	N/A	-2.5	NC	TAľ
145+00	-3.0	-3.0		-3.1	-3.1	N/A	-2.5	NC	

Design Exception - Superelevation Rate I-81 NB Truck Climbing Lane UPC 116156

a	Inside (Left) Lane			Outside (Right) Lane		Truck Climbing Lane		VDOT/AASHTO	
Station	Existing	Proposed		Existing	Proposed	Existing	Proposed	(2011) Standard ¹	
145+50	-3.6	-3.6		-2.9	-2.9	N/A	-2.5	NC	A
146+00	-2.1	-2.1	1	-2.7	-2.7	N/A	-2.5	NC	
146+50	-3.0	-3.0	1	-2.8	-2.8	N/A	-2.5	NC	FICI
147+00	-3.1	-3.1		-3.0	-3.0	N/A	-2.5	NC	OF
147+50	-2.2	-2.2		-3.0	-3.0	N/A	-2.5	NC	3AL
148+00	-2.4	-2.4		-2.5	-2.5	N/A	-2.5	NC	NTF
148+50	-2.1	-2.1		-1.7	-1.7	N/A	-2.5	NC	Ë
149+00	-2.1	-2.1		-2.5	-2.5	N/A	-2.5	NC	
149+50	-2.7	-2.7		-2.1	-2.1	N/A	-2.5	NC	VC STE
150+00	-2.7	-2.7		-2.2	-2.2	N/A	-2.5	NC	UE BY
150+50	-3.0	-3.0		-2.4	-2.4	N/A	-2.5	NC	DED
151+00	-3.5	-3.5		-3.4	-3.4	N/A	-2.5	NC	DT F
151+50	-2.0	-2.0		-1.3	-1.3	N/A	-2.5	NC	PRC
152+00	-2.8	-2.8		-1.1	-1.1	N/A	-2.5	NC	ION
152+50	-3.1	-3.1		-1.8	-1.8	N/A	-2.5	NC	ANC
153+00	-3.0	-3.0		-2.3	-2.3	N/A	-2.5	NC	ER/ XCE
153+50	-3.0	-3.0		-3.5	-3.5	N/A	-2.5	NC	D N E
154+00	-2.4	-2.4		-2.6	-2.6	N/A	-2.5	NC	L NI SIG
154+50	-2.9	-2.9		-3.0	-3.0	N/A	-2.5	NC	DE DE
155+00	-2.6	-2.6		-3.1	-3.1	N/A	-2.5	NC	20 K
155+50	-2.1	-2.1		-2.5	-2.5	N/A	-2.5	NC	0PE /20:
156+00	-1.6	-1.6		-2.1	-2.1	N/A	-2.5	NC	//8/
156+50	-2.9	-2.9		-2.9	-2.9	N/A	-2.5	NC	OSS I
157+00	-2.0	-2.0		-2.9	-2.9	N/A	-2.5	NC	CR(ATE
157+50	-2.1	-2.1		-2.3	-2.3	N/A	-2.5	NC	L D
158+00	-1.9	-1.9	L R	-2.5	-2.5	N/A	-2.5	NC	NAI
158+50	-1.2	-1.2	Å.	-2.3	-2.3	N/A	-2.5	NC	EI
159+00	-2.0	-2.0	I Z	-3.3	-3.3	N/A	-2.5	NC	Z Z
159+50	-2.2	-2.2	SEL	-3.6	-3.6	N/A	-2.5	NC	0L
160+00	-1.1	-1.1	BA	-1.8	-1.8	N/A	-2.5	NC	SEC.
160+50	-1.7	-1.7		-2.7	-2.7	N/A	-2.5	NC	LT (
161+00	-2.5	-2.5		-2.5	-2.5	N/A	-2.5	Transition	GE
161+50	-1.8	-1.8		-2.3	-2.3	N/A	-2.5	Transition	AN
162+00	-1.0	-1.0		-3.2	-3.2	N/A	-2.4	Transition	F
PC 162+34.08									
162+50	0.6	0.6		-2.4	-2.4	N/A	-2.4	Transition	F
163+00	0.7	0.7	4	-3.3	-3.3	N/A	-3.3	-4.0	IGN
163+50	0.7	0.7	4	-3.2	-3.2	N/A	-3.2	-4.0	EQ
164+00	1.6	1.6	-	-3.0	-3.0	N/A	-3.0	-4.0	N R
164+50	1.0	1.0	-	-3.1	-3.1	N/A	-3.1	-4.0	TIO TIO
165+00	0.8	0.8		-3.2	-3.2	N/A	-3.2	-4.0	UN U
165+50	1.6	1.6		-4.2	-4.2	N/A	-4.2	-4.0	EXC
166+00	1.6	1.6	-	-3.3	-3.3	N/A	-3.3	-4.0	
166+50	2.1	2.1	_	-2.7	-2.7	N/A	-4.1	-4.0	â
167+00	0.7	0.7	_	-4.8	-4.8	N/A	-4.8	-4.0	NE STEI
167+50	1.1	1.1	4	-4.5	-4.5	N/A	-4.5	-4.0	CE9
168+00	0.7	0.7	-	-4.1	-4.1	N/A	-4.1	-4.0	. RE
168+50	1.9	1.9	-	-2.3	-2.3	N/A	-4.7	-4.0	(IST JOT
169+00	0.8	0.8	-	-5.3	-5.3	N/A	-5.3	-4.0	ION ION
169+50	1.4	1.4	-	-4.3	-4.3	N/A	-4.3	-4.0	R TC
1/0+00	1.2	1.2	-	-4.5	-4.5	N/A	-4.5	-4.0	PEF
170+50	1.1	1.1	-	-3.5	-3.5	N/A	-3.5	-4.0	TA N E
1/1+00	1.7	1.7	-	-4.0	-4.0	N/A	-4.0	-4.0	-003 1003
1/1+50	1.6	1.6	-	-4.1	-4.1	N/A	-4.1	-4.0	و DE
1/2+00	0.7	0.7	1	-3.2	-3.2	N/A	-3.2	-4.0	

1. AASHTO - A Policy on Geometric Design of Highways and Streets (2011), Section 3.3.5, Table 3-10b

VDOT Road and Bridge Standards (2016), Section 800, Page 803.43

* % cross slope (+/-) is NB direction of travel with baseline POFG

Based on Public Hearing cross sections dated March 13, 2021 (Survey Updated July 2020)

Design Exception – Superelevation Rate I-81 NB Truck Climbing lane UPC 116156

Cost to meet minimum AASHTO Standard:

The improvements to I-81 Northbound that would be necessary to achieve the AASHTO minimum standard for superelevation include but are not limited to earthwork, milling and overlay of the existing lanes beyond the project limits, cross slope correction of the existing lanes through the project limits via asphalt overlays, guardrail replacement, and signing/pavement marking beyond the project limits. A detailed cost estimate of the additional impacts to the project are estimated as follows:

	\$	52,500
ONS @ \$110/TON =	\$	690,250
4,250 SY @ \$25/SY =	\$	356,250
	\$	162,450
:	\$	31,540
	\$	63,080
	\$	31,540
	\$	252,290
	\$	31,540
=	\$	138,600
	\$	167,145
Total	\$	1,977,185
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Are there any plans to improve the approach roadway within the next ten (10) years?

No

Accident history for the past 3 years:

Safety considerations associated with substandard superelevation rates on a freeway are collisions resulting from skidding through a curve and resulting in a run-off-road crash. Crash data was obtained along the project limits for the three-year period from July 1, 2017 to June 30, 2020. Please refer to Attachment D for a map depicting the crash locations and a chart providing details about the crashes.

There were a total of 11 crashes within the project limits with a type distribution shown in the table below. Over half (7 crashes) were single vehicle run-off-the-road crashes. Five of the single vehicle run-off-theroad crashes occurred during a rain event, and one occurred with oil or other fluids located on the road surface.

I-81 Northbound Prevalent crash type					
1. Rear End	2				
2. Angle	1				
4. Sideswipe – Same Direction	1				
9. Fixed Object – Off Road	7				
TOTAL	11				

Design Exception – Superelevation Rate I-81 NB Truck Climbing lane UPC 116156

Effect of design exception on safety:

Since superelevation on the mainline is an existing condition, no adverse impact on the existing capacity and operation of the facility is anticipated if the existing superelevation is maintained.

Any mitigation of the substandard design element(s):

The proposed design will incorporate 6-inch pavement markings and shoulder rumble strips. VDOT Bristol District Traffic Personnel ball banked each curve within the project limits and determined that no additional curve warning signs or advisory speeds were warranted. A copy of the email with the District Traffic Engineer's recommendations is included Appendix E.

Will the exception affect the other controlling criteria?

This request for an exception is for existing substandard superelevation and is not anticipated to have any impact on other controlling criteria.

Attachments:

- A. Original Plan Sheets and 1953 Road Design Standards
- B. HB 865
- C. Title Sheet, Typical Section Sheet, Plan & Profile Sheets, Cross Section Sheets
- D. 3-year Crash Summary
- E. District Traffic Engineer Concurrence Emails
Attachment A

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EXPLANATION OF TABLES AND INSTRUCTIONS FOR THEIR USE

These tables contain superelevation and widening corrections for all standard pavement widths through a range of curves considered most likely to be used in highway design. Furthermore, a range of transition lengths is provided to afford the locating or designing engineer an appreciable degree of flexibility in fitting his alignment to various conditions. In all cases the longest possible transition (Ls) shall be used.

On sheet 87 will be found the standard symbols used throughout these tables.

On sheet 93 is shown a summary of the range of standard pavement widths (W), transition lengths (Ls), and degrees of curve (Dc) covered herein, together with the maximum widening (w), superelevation rate (e), and the approximate maximum safe speeds (V) afforded thereby.

The number of corrections to be applied is three for each of ten uniformly spaced positions on each transition. The location center line is shifted toward the center of the curve the distances Zc to produce the spiral-like transition. The distances Zt are laid off right and left of the shifted center line to establish the edges of the widened pavement (Zt includes the widening and is identical for both sides), and the outer edge is raised above the location center line grade and the inner edge depressed below the location center line grade the amount S (identical for both edges of pavement). See sheets 90 and 91 for graphical illustrations of the application of these corrections.

To determine the sq. yds. of additional pavement for widening, multiply the figure in the column headed "Sq. Yds. Add. Pave. Per 100 Ft. of Curve" by the entire length of the location center line curve in stations as measured from the P.C. to the P.T.

See sheet 91 for the treatment of curves of less than 2 degrees.

The distance between curves in <u>opposite</u> directions shall be as long as possible and at least long enough to permit using the shortest transition length (Ls) set up in these tables for each of the two curves.

The distance between curves in the <u>same</u> direction shall preferably be long enough to avoid a "broken back" appearance, and at least long enough to permit using the shortest transition length (Ls) set up in these tables for each of the two curves. Curves too close to-gether to meet the latter requirement shall be compounded. Compound curves shall be transitioned at the P.C. and P.T., and a transition at the P.C.C., if necessary, will be designed in the office.

For odd degree curves use the transitions and corrections for the nearest whole or half degree curve found in the tables.

On curves from 2° to 6° inclusive, no widening is to be applied on 24' pavements.

For curves sharper than 30° the tabulated values for 30° curves shall be used.

For minimum design factors for various design speeds see sheet 94.

The longest possible transition shall be used and the length noted along with the curve data, for example, Ls = 300. It should be noted, however, that no transition can be longer than the curve to which it is applied. The standard shift (Zc) shall be kept in mind for its effect on clearance.

(continued)

88

TC-2

VIRGINIA	
DEPARTMENT_OF	HIGHWAYS
NOVEMBER I,	1953



Attachment B

2010 SESSION

VIRGINIA ACTS OF ASSEMBLY -- CHAPTER

An Act to amend and reenact § **46.2-870** of the Code of Virginia, relating to maximum highway speed limits. [H 856] Approved

Be it enacted by the General Assembly of Virginia:

1. That § **46.2-870** of the Code of Virginia is amended and reenacted as follows:

§ 46.2-870. Maximum speed limits generally.

Except as otherwise provided in this article, the maximum speed limit shall be 55 miles per hour on interstate highways or other limited access highways with divided roadways, nonlimited access highways having four or more lanes, and all state primary highways.

The maximum speed limit on all other highways shall be 55 miles per hour if the vehicle is a passenger motor vehicle, bus, pickup or panel truck, or a motorcycle, but 45 miles per hour on such highways if the vehicle is a truck, tractor truck, or combination of vehicles designed to transport property, or is a motor vehicle being used to tow a vehicle designed for self-propulsion, or a house trailer.

Notwithstanding the foregoing provisions of this section, the maximum speed limit shall be 65 70 miles per hour where indicated by lawfully placed signs, erected subsequent to a traffic engineering study and analysis of available and appropriate accident and law-enforcement data, on: (i) interstate highways, (ii) multilane, divided, limited access highways, and (iii) high-occupancy vehicle lanes if such lanes are physically separated from regular travel lanes. The maximum speed limit on Interstate Route 85 shall be 70 miles per hour where indicated by lawfully placed signs, erected subsequent to a traffic engineering study and analysis of available and appropriate accident and law-enforcement data. The maximum speed limit shall be 60 miles per hour where indicated by lawfully placed subsequent to a traffic engineering study and analysis of available and appropriate accident and law-enforcement data. The maximum speed limit shall be 60 miles per hour where indicated by lawfully placed signs, erected subsequent to a traffic engineering study and analysis of available and appropriate accident and law-enforcement data. The maximum speed limit shall be 60 miles per hour where indicated by lawfully placed signs, erected subsequent to a traffic engineering study and analysis of available and appropriate accident and law-enforcement data, on U.S. Route 29, U.S. Route 58, U.S. Route 360, U.S. Route 460, and on U.S. Route 17 between the town of Port Royal and Saluda where they are nonlimited access, multilane, divided highways.

Attachment C

3/11/2021 12:09:12 PM

LINU		FOR INDEX OF SHEETS SEE SHEET 1B
DESIGN		THIS PROJECT WAS DEVELOPED UTILIZING THE DEPARTMENT'S ENGINEERING DESIGN PACKAGE OPENROADS/GEOPAK. GEOPAK Computer Identification No. UPC 116156
DISTRICT		PLA
BRISTOL	7_(Bristal) (Nov.24,2019) Bristol)	Description Reference Mile Mark 32,197 1-81 NB
	IAGER James_Jones.L.S.(276).696-325 , DATE Woolpert Inc., (757).549-3549 mara_Pritchard, P.E. (276).696-3249.11 UTILITY BY, DATE 	STATE LINE CONVENTIONAL SIGNS STATE LINE COUNTY LINE COUNTY LINE COUNTY LINE COUNTY LINE COUNTY LINE RIGHT OF WAY LINE PENCE LINE UNFENCED PROPERTY LINE WATER LINE SANITARY SEWER LINE CASE CONSURVEY LINE CASE CONSURVEY LINE CASE CONSURVEY LINE CASE CONSULTANT CONSULVANT CONSULATION CONSULTANT CONSULTAT
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		THE COMPLETE ELECTRONIC PDF VERSION OF THE PLAN ASSEMBLY AS AWARDED, HAS BEEN <u>SEALED AND SIGNED</u> USING DIGITAL SIGNATURES AND THE OFFICIAL PLAN ASSEMBLY IN ELECTRONIC FORMAT IS STORED IN THE VDOT CENTRAL OFFICE PLAN LIBRARY, INCLUDING ALL SUBSEQUENT REVISIONS, WILL BE THE OFFICIAL CONSTRUCTION PLANS. FOR INFORMATION RELATIVE TO ELECTRONIC FILES AND LAYERED PLANS, SEE THE GENERAL NOTES. DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT.
		THIS PROJECT IS TO BE CONSTRUCTED IN ACCORDANCE WITH THE DEPARTMENT'S 2020 ROAD AND BRIDGE SPECIFICATIONS, 2016 ROAD AND BRIDGE STANDARDS, 2009 MUTCD, 2011 VIRGINIA SUPPLEMENT TO THE MUTCD, 2011 VIRGINIA WORK AREA PROTECTION MANUAL AND AS AMENDED BY CONTRACT PROVISIONS AND THE COMPLETE ELECTRONIC PDF VERSION OF THE PLAN ASSEMBLY.Washingto Washingto State NO.
		ALL CURVES ARE TO BE SUPERELEVATED, TRANSITIONED AND WIDENED IN ACCORDANCE WITH STANDARD TC-5.11, EXCEPT WHERE OTHERWISE NOTED. THE <u>ORIGINAL</u> APPROVED TITLE SHEET(S), INCLUDING ORIGINAL SIGNATURES, ARE FILED IN THE VDOT CENTRAL OFFICE PLAN LIBRARY.
		ANY MISUSE OF ELECTRONIC FILES, INCLUDING SCANNED SIGNATURES,



COMMONWEALTH OF VIRGINIA

DEPARTMENT OF TRANSPORTATION

AN AND PROFILE OF PROPOSED STATE HIGHWAY

WASHINGTON COUNTY

I-8I NORTHBOUND TRUCK CLIMBING LANE FROM: MILE MARK 32.197 TO: MILEMARK 33.519



ton County Population 54,402 (2018 Census)

SECTION	FEDERAL AID	FEDERAL AID PROJECT NO.	TYPE	UPC	EQUALITIES	LENGTH I BRID(LENGTH INCLUDING BRIDGE(S)		Excluding Ge(S)		DE
			CODE	NO.	FEET	FEET	MILES	FEET	MILES		
	PE-101	NHPP-081-1 (362)		116156		6,980.00	1.322	6,980.00	1.322	PRELIM. ENGR.	Fr: MILE MARK To: MILE MARK
	RW- 201			116156		5,048.30	0.956	5,048.30	0.956	RIGHT OF WAY	Fr:MILE MARK To:MILE MARK
	C-501		1000	116156		5,400.00	1.023	5,400.00	1.023	CONSTR.	Fr: MILE MARK To: MILE MARK
	Project Ler	ngths are bas	sed on	1-81 NE	BL Constructi	ion Baselin	<i>ie.</i>				

FHW

dll6l5600l.dgn Plotted By:elilley

	LIM	ITED ACCES	S HIGHWAY	 By Reso dated C 	olution of Highway Commissio October 4, 1956	n	
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		" (SEE TAB FOR SEC	ULATION BELON TION NUMBERS		PE-IOI, RW-2 (SEE TARUI ATION	:UI,C-501 BELOW	
					FOR SECTION NU	IMBERS)	
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	To: M	ILE MARK 3	53.519				
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ADT (2045)	21,404	1					
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T (%) (design hour)	17%						
V (MPH)	See Pla	an and Profil	e Sheets for	Horizont	al and Vertical Curve	Design Spec	ed
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					PROJECT	-829 ^s	HEET NO.





UNIT

DESIGN

DISTRICT

BRISTOL

d116156002a(1).dgn

Plotted By: elilley LIMITED ACCESS HIGHWAY By Resolution of Highway Commission dated October 4 1956

					00100 0010001 4, 1930	
		REVISED	STATE		STATE	
			STATE	ROUTE	PROJECT	SHEET NO.
			VA.	81	0081-095-829, RW-201 C-501	2A(I)
aterials	VDOT Location & Design	DESIGN FEATU OR TO REGULA MAY BE SUBJE NECESSARY BY	RES RELA ITION ANE ECT TO C ' THE DEI	ATING TO CONTR HANGE A PARTMEN) CONSTRUCTION OL OF TRAFFIC AS DEEMED AT	
Irginia ENGINEER	ROADWAY ENGINEER					

INSET B SHOULDER-TRUCK – (2)CLIMBING LANE 5 3 (4)

LEGEND

- ()ASPHALT CONCRETE SURFACE COURSE, TYPE SM-12.5E @ 220 LBS. PER S.Y.
- \bigcirc ASPHAL CONCRETE INTERMEDIATE COURSE, TYPE IM-19.0E @ 230 LBS.PER SQ YD
- IO" ASPHALT CONCRETE BASE COURSE, TYPE BM-25.0A $(\overline{\mathcal{Z}})$
- (4)** IO" AGGREGATE BASE MATERIAL, TYPE I, NO. 21B
- (5) 12" AGGREGATE MATERIAL NO.1

** 2" OF THE IO" IS FOR LEVELING COURSE OVER AGGREGATE MATERIAL NO.I

	PROJECT	SHEET NO.
	0081-095-829	2A(1)

Utility Owners:

Electric:

Appalachian Power Scarlet Collie 13563 Owens Drive Glade Spring, VA 24340 (276) 429-4117

Communication:

Point Broadband Tim Nutter 15022 Lee Highway Bristol,VA 24202 (276) 492-1953

CenturyLink Steve Hastings 175 South 1st Street Wytheville, VA 24382 (276) 223-6264

Water:

Washington County Service Authority Kirk Maiden 25/22 Regal Drive Abingdon, VA 242// (276) 676-6773





d116156003**.**dgn Plotted By: elilley I IMITED ACCESS HIGHWAY By Resolution of Highway Commission

		LIVITED AC	,UE33 F		dáted October 4, 1956	
		REVISED STATE STATE				
				ROUTE	PROJECT	ONEET NO.
			VA.	81	0081-095-829,PE-101; RW-201,C-50F	3
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on & Design ⁄irginia ENGINEER	VDOT Location & Design Bristol, Virginia ROADWAY ENGINEER					

ROADWAY ITEMS LEGEND
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2 ST'D.GR-MGS2 REQ'D.
\int_{3} ST'D.GR-MGS3 REQ'D.
$\Delta f'D.GR-MGS4 REQ'D.$
5 REMOVE EXISTING GUARDRAIL
46 ST'D.GR-10 TYPE II REQ'D.
Z7 ST D.GR-MGSI, LONG POST RECTD.
ST'D.MC-3B, ASPHALT CURB AND ASPHALT CONCRETE CURB BACKUP MATERIAL REQ'D.
ST'D.UD-4 UNDERDRAIN REO'D.
ST'D.UNDERDRAIN INSPECTION
CONSTRUCTION CLIMITS IN CUTS
ECONSTRUCTION
XXX PARCEL NUMBER
X-X DRAINAGE STRUCTURE
PROPOSED PAVEMENT
PROPOSED FLEXIBLE PAVEMENT
DEMOLITION OF PAVEMENT
REFERENCES
(PROFILES, DETAIL & DRAINAGE DESCRIPTION SHEETS, ETC.)
Alignment Data IG

	SCALE		PROJECT	SHEET NO.
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3/12/2021 10:17:21 AM

> SURVEYED BY, DATE <u>Woolpert Inc., (757) 549-3549 (Nov.24,2019)</u> DESIGN BY Tamara Pritchard, P.E. (276) 696-3249 (Bristol)



d116156004.dgn

 Plotted By: elilley

 LIMITED ACCESS HIGHWAY

 By Resolution of Highway Commission

 dated October 4, 1956

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		REVISED STATE STATE				
			STATE	ROUTE	PROJECT	SHEET NO.
			VA.	81	0081-095-829, PE-101; RW-201, C-501	4
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on & Design /irginia ENGINEER	VDOT Location & Design Bristol, Virginia ROADWAY ENGINEER					

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PROJECT MANAGER <u>James Jones, L.S. (276) 696-3257</u> (Bristol) SURVEYED BY, DATE <u>Woolpert Inc.</u> (757) 549-3549 (Nov. 24, 2019) DESIGN BY <u>Tamara Pritchard, P.E. (276) 696-3249</u> (Bristol) SUBSURFACE UTILITY BY, DATE _____ PH PLANS THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY. 2,045 2,040 2,035 2,030 2,025 2,020 2,015_ 2,010_ 2,005 2,000_ 1,995 1,990 1,985 1,980 1,975 1,970_ 1,965 1,960_ $-\infty$ $\mathbf{\alpha}$ 1,955 113+00.00 114+00.00 115+00**.**00



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Plotted By: elilley

LIMITED ACCESS HIGHWAY	By Resolution of Highway Commissio dated October 4, 1956
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PROJECT MANAGER James Jones, L.S. (276) 696-3257 (Bristol) SURVEYED BY, DATE _ Woolpert Inc., (757) 549-3549 (Nov.24,2019) DESIGN BY _ Tamara _ Pritchard, P.E. (276) 696-3249 (Bristol) SUBSURFACE UTILITY BY, DATE _____ PH PLANS THE SEPLANSAREUNFINISHEDANDUNAPPROVEDANDARENOTTOBEUSEDFORANYTYPEOFCONSTRUCTIONORTHEACQUISITIONOFRIGHTOFWAY. 2,080 2,075 2,070 2,065 2,060 2,055 2,050 2,045 2,040 2,035 2,030 4 4 4 2,025 ΞĿ SHI 2,020 50.00 2,015_ NE 2,010 CH CH MAT 2,005 $\prec \prec$ 2,000_ 1,995 2.022.95 2.02432 2.020.27 2.021.59 2.0/8.89 5.6/ 17.57 602 00.0 1,990

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DESIGN

DISTRICT

BRISTOL

ROJECT MANAGER _James_Jones,L.S. (276) 696-3257 (Bristol) SURVEYED BY, DATE _<u>Woolpert Inc., (757) 549-3549 (Nov.24,2019)</u>_____ DESIGN BY <u>Tamara Pritchard, P.E. (276) 696-3249 (Bristol)</u> SUBSURFACE UTILITY BY, DATE _____. PH PLANS THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY. S Interstate 81 (Southbound) L S H Concrete S 8 Î 12" (LANE / 🕂 12' LANÉ 🗡 🕂 12'LANE 🔶 IO' SHOULDER · - < − 15' <u>[[]4</u>] 6-2 |\/8\ 6-4 6-1 N -6-3 - EXISTING WATER LINE SHALL BE LOCATED BY THE CONTRACTOR AND SHALL NOT BE DISTURBED. 2035. 2035. ROY WAYNE STEVENS ~ 5 J ANITA S. STEVENS In. 4 Inv. Inst.# 080003741 197.11 AC. Per Assessor's Office TAX MAP# 071-A-8A2 ROY WAYNE STEVENS & ANITA SHARON STEVENS D END No Deed Reference 197.11 AC. Per Assessor's Office TAX MAP* 071-A-8A2 HYDRAULICS NOTES <u>/H2</u>

REMOVE EXISTING EW-12 STRUCTURE DAYLIGHT CROSS DRAIN / UNDER DRAIN INSTALL PROPOSED STANDARD EW-12 <u>/H</u>A REMOVE EXISTING EW-12.CONNECT CROSS DRAIN / UNDER DRAIN TO PROPOSED STRUCTURE HS EXISTING 24" CM PIPE TO BE REMOVED

8:48:24 AM



d116156006.dgn LIMITED ACCESS HIGHWAY By Resolution of Highway Commission

				- · ·	dated Uctober 4, 1956			
		REVISED	STATE		STATE			
			STATE	ROUTE	PROJECT	SHEET NO.		
			VA.	81	0081-095-829,PE-101; RW-201,C-50F	6		
		DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT						
on & Design ⁄irginia ENGINEER	VDOT Location & Design Bristol, Virginia ROADWAY ENGINEER							

0	50'	100'	0081-095-829

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PROJECT MANAGER James Jones, L.S. (276) 696-3257 (Bristol) SURVEYED BY, DATE _ Woolpert Inc., (757) 549-3549 (Nov.24,2019) DESIGN BY _ Tamara_Pritchard, P.E. (276) 696-3249 (Bristol) SUBSURFACE UTILITY BY, DATE _____ PH PLANS THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY. 2,130 2,125 2,120 2,115 2,110 2,105 2,100 2,095 2,090 2,085 2,080 2,075 2,070 54 2,065 SHE 2,060 50.00 137 2,055_ マ ST 4/NE 2,050 MATCH 2,045_ 2.059.92 2.062.05 2.066.38 2.066.38 2.070.32 2.070.32 2.064.26 2.064.26 <u>6826</u> 68.26 2.055.9/ 2.057.9/ 2,040_

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ROJECT MANAGER _James_Jones,L.S. (276) 696-3257 (Bristol) SURVEYED BY, DATE _<u>Woolpert Inc., (757) 549-3549 (Nov.24,2019)</u>_____ DESIGN BY <u>Tamara Pritchard, P.E. (276) 696-3249 (Bristol)</u> SUBSURFACE UTILITY BY, DATE _____. PH PLANS THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY. Interstate 81 (Southbound) $\boldsymbol{\mathcal{C}}$ ______ 55 <u>Concrete Swale</u> 5 2 _____ 12' LANE 🔶 (12' LANE/ 👾 XIŽ LANĖ 🔶 🔶 10' SHOULDER Š OBSCURED Ň LBEGIN RW-3 ST A. 151.00.00 JOY BAPTIST CHURCH Inst.# 000033940 10.69 AC. Per Assessor's Office TAX MAP* 053-A-30



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					dated Uctober 4, 1956			
		REVISED	STATE		STATE			
			STATE	ROUTE	PROJECT	SHEET NO.		
			VA.	81	0081-095-829,PE-101; RW-201,C-50i	7		
		DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED						
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PROJECT MANAGER James Jones, L.S. (276	(6)_696-3257_(Bristol)			DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE	SHEET NO
SURVEYED BY, DATE <i>Woolpert Inc.,(757)5</i> DESIGN BY <i>Tamara Pritchard, P.E.(276)</i> 69 SUBSURFACE UTILITY BY, DATE	549-3549 (Nov.24,2019) 96-3249 (Bristal)		CRUSS SECTIONS scale 1 in. = 10 ft	OR TO REGULATION AND CONTROL OF TRAFFIC ROUTE PROJECT MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT	SHELTNO.
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PROJECT SHEET NO.			1.01	9				
9							PROJECT SH 0081-095-829	IEET NO. 9

CROSS SECTIONS	N REVISED STATE	STATE ROUTE PROJECT	SHEET NO.		
SCALE 1 IN. = 10 FT		NECESSARY BY THE DEPARTMENT		81 0081-095-829,C-5	501 10
				2060	
9. 66.02	2039.3 2038.83 2038.53				
<u>17</u> . <u>7</u> 2.	75' EL 75' EL			2050	
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		L 2019.1	76' RT 215.73 (23	10. 2030 2030	
		EXIST	ET 2013		
$+3\cdot3+00\cdot00$			H 108.9 113.93	2020	
	7.81				
<u>EL 20</u> 3	EL 203 EL 2037			2050	
MATCH MATCH	22.60' 38.60' 38.60'			2040	
	-5.0%	g		₩	.95
		<u>EL 2016</u> 94.11' RT	5.37 ⁴ .R7	977107 977107 1077107 1077107	E1 2015
			TEMP ESMT	EXIST & II II R/W 0 2020	149.36*
132+50.00				137	
<u> </u>	50	Station 132+50.00 To	o Station 133+ o	00.00	
				PROJECT 0081-095-829	SHEET NO. 10

PROJECT MANAGER James Jones, L.S. (276)-696-3257_(Bristol) SURVEYED BY, DATE Woolpert Inc., (757) 549-3549 (Nov. 24, 2019)	CROSS	SFCTIONS	DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE ROU	STATE SHEET NO.
DESIGN BY <i>Tamara_Pritchard, P.E. (276) 696-3249 (Bristol)</i> SUBSURFACE UTILITY BY, DATE	SCALE 1	IN. = 10 FT	MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT	
PH PLANS				0001-093-029,0-301 11
TO BE USED FOR ANY TYPE			4	
2060 ACOUISITION OF RIGHT OF WAY.		2044	2043. 2043.	2060
2050		95' El	5. 5. 2.20. H	2050
		MATCH MATCH S EXIST. EXIST257	EXIST	
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		131-5000	85.41 33.	
		1.5.4' JO.OO		
2060		43.02		2060
		-1 20r	Ľ 204 RT	
2050		2.38' 1	93.43 F	2050
		MATCH MATCH ₹ N EXIST. EXIST2.5% -5.0	EXIST	
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			3' El 0034'	
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2050		EF 52		2050
		МАТСН МАТСН 78. 10. МАТСН	32.84 38.84 93.6	
2040		EXIST. EXIST2.5% -5.0	$\frac{E \times 15}{L/A}$	2040
			2018.8 2018.8	
20.30			205 E	2030
2020			68.46 6. 63.3	$2\dot{D}$
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			Station 133+50.00 To Station 134+50.0	20
100 100	50 		50	PROJECT /50
				0081-095-829
PROJECT MANAGER James Jones, L.S. (276) 696-3257_(Bristol)			DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE STATE STATE STATE	
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SURVEYED BY, DATE Woolpert Inc., (257) 549-3549 (Nov. 24, 2019). DESIGN BY Tamara Pritchard, P.E. (276) 696-3249 (Bristol) SUBSURFACE UTILITY BY, DATE	CRUSS scale 1	SECIIUNS $IN. = 10 FT$	OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT	
PH PLANS			VA. 81 0081-095-829,C-501 12	
THESE PLANS ARE UNFINISHED				
TO BE USED FOR ANY TYPE				
ACOUISITION OF RIGHT OF WAY.				
		048.15	25.57 20.74 20.74	
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$2\dot{D}5\dot{D}$		MATCH MATCH	\vec{x}	
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2060		EF 50	$\begin{array}{c c} 77 \\ \hline 007 \\ \hline 77 \\ \hline 7 \hline 7$	
2050			11 20 20 20 20 50 20 50	
		EXIST. EXIST2.5% -5.0%		
2040				
2030				
		135+00.00		
100	50		Station 135+00.00 To Station 135+50.00	
		$\mathbf{\Psi}$		

PROJECT MANAGER <i>James Jones, L.S. (276)-696-3257_(Bristol)</i> SURVEYED BY, DATE <i>Woolpert Inc., (757) 549-3549 (Nov. 24, 2019)</i> DESIGN BY <i>Tamara_Pritchard, P.E. (276) 696-3249 (Bristol)</i>		SECTIO	DNS	DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED AND CONTROL OF TRAFFIC STATE ROUTE PROJECT
SUBSURFACE UTILITY BY, DATE PH PLANS	SCALE I			NECESSARY BY THE DEPARTMENT VA. 8/ 0081-095-829,C-50/ /3
THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE			<u>با</u>	
2070 OF CONSTRUCTION OR THE ACOUISITION OF RIGHT OF WAY.			1. 2053.5	2070 2070 202 2 7
2060		MATCH M EVIST E	NATCH	11 12 12 12 12 12 12 12 12 12 12 12 12 1
2050			<u>-2.5%</u> -5.0	<u>07.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>17.</u> <u>1</u>
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		<u> </u>	00	
207.0			050.04	2070 72-67 66 67
2060			23' EL 2	2060 11 2060
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				$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$
20.30		1.36+000	00	
				Station 136+00.00 To Station 137+00.00
100 50		<i>•</i>		50 100 150 PROJECT SHEET NO. 0081-095-829 13



CROSS SECTIONS		DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED	REVISED	ROUTE	STATE PROJECT	
SCALE 1 IN. = 10 FT		NECESSARY BY THE DEPARTMENT	VA	. 81	0081-095-829 , C-501	14
257.67	56.37 56.77 56.57					
上 「 上 「 」	EL 20 EL 20				2070	
MATCH MATCH	23.77 35.77 39.77	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			2060	
		2038.9			1.25' RT	
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158+00.00						
5025,69	054.59 054.59 054.59				2070	
	L EL 2 L EL 2 L EL 2					
MATCH MATCH S EXIST. EXIST2.5	999 88 88 50 7 5.0%	N.			2060	
		-1.1 2031 			2050	
		EXIST			EXIST R/W	
137+50.00						
50		Station 137+50.00 To	Station 138	•00.00		
$\boldsymbol{\varphi}$					PROJECT 0081-095-829	SHEET NO. 14

PROJECT MANAGER James Jones, L.S. (276) 696-3257_(Bristol) SURVEYED BY, DATE Woolpert Inc., (757) 549-3549_(Nov. 24, 2019)	CROSS SECT	DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE OR TO REGULATION AND CONTROL OF TRAFFIC ROUTE PROJECT SHEET NO.
DESIGN BY <i>Tamara_Pritchard, P.E. (276) 696-3249 (Bristol)</i> SUBSURFACE_UTILITY_BY, DATE	SCALE 1 IN. = 10	MAY BE SUBJECT TO CHANGE AS DEEMED 10 FT NECESSARY BY THE DEPARTMENT VA 8/ 008/-095-829 C-50/ /5
PH PLANS		
THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT		
OF CONSTRUCTION OR THE		00.00 00.000000
		EL 200
2070	ΜΔΤΟΗ	$\frac{2070}{1}$
		<u>EX/S7.</u> -2.5% -5.0% <u>S</u>
2050		$\frac{1}{2} = \frac{1}{2} = \frac{1}$
2040	139+	9+50.00 2040
2080		8000 2080 2080
		L 2060
2070		
2060	MAI CH EXIST.	$\frac{MATCH}{EXIST.} = \frac{2.5\%}{-5.0\%} = \frac{1}{2060}$
20.50		$\frac{1}{2050}$
	/39+(9+00.00
2080		
2070		502 7 7 7
2060	MATCH EXIST.	$\frac{MATCH}{EXIST} \approx \frac{-2.5\%}{2060} \approx \frac{-5.0\%}{2060}$
2050		2050 E XIST
2040		6 L/A R/W 2040
	138+	
150 100	50	φ STOTION 138+5U.UU 1 0 STOTION 139+5U.UU 0 STOTION 139+5U.UU 0 STOTION 139+5U.UU 1 150
		PROJECT SHEET NO. 0081-095-829 15

PROJECT MANAGER <u>James Jones, L.S. (276) 696-3257_(Bristol) _</u> SURVEYED BY, DATE Woolpert Inc., (757) 549-3549 (Nov. 24, 2019) DESIGN BY Tamara Pritchard, P.F. (276) 696-3249 (Bristol)	CROSS SECTIONS	DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE SHEET NO. OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED STATE
SUBSURFACE UTILITY BY, DATE	SCALE 1 IN. = 10 FT	NECESSARY BY THE DEPARTMENT VA. 8/ 008/-095-829.C-50/ /6
AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE		
ACOUISITION OF RIGHT OF WAY.		
	67.95 5.85 6.85	
2080	EL 200 EL 200 EL 200	2080 S
2070	$MATCH \qquad MATCH \qquad \overset{\ref{eq:main stars}}{\overset{\ref{eq:main stars}}{\re$	E_X/ST L/A $\frac{2070}{5}$
		2049. 2049.2 37.59' R
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2050		2050
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2080	E 2065. 2065.	2080
2070	MATCH MATCH	2070
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	140+00.00	
		Station 140+00.00 To Station 140+50.00
150 100 50	ϕ	100 PROJECT SHEET NO.
		<i>0081-095-829 16</i>



CROSS SECTI	nns	DESIGN FEATURES RELATING TO CONSTRUCTION OR TO REGULATION AND CONTROL OF TRAFFIC	REVISED STATE	STATE SHEET NO.
SCALE 1 IN. = 10 F	- T	MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT	VA.	81 0081-095-829,C-501 17
	074.02 773.72 773.72	76 .77		2090
	EL 20 EL 20 EL 20	EL. 20		
MATCH	24.65 34.65 Заб55 С. 12.65 С.	40.65 9.07 RT RT		2080 §
	<u>EXISI:</u> -2.5% -5.0%	202 71.92 71.92		
		28.84 6.038 6.51 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7		
142+01	$\mathcal{O}\mathcal{O}\mathcal{O}$			2060
	2071.95 071.15 071.15	C 270 / C		2090
	L C C C C C C C C C C C C C C C C C C C			2080
MATCH EXIST.	MATCH EXIST: -254	5.85 5.32 40.51		45.8% R
		EL 205 2053. 2053.		ZOŽBT R/W
				2080
141+50	<i>J.OO</i>			
				2090
	2069.61 2069.01 2069.01	008:30		
				2080
MATCH EXIST.	MATCH 2 EXIST2.5% -5.0%	2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0		E 2070
		205. 1.95% 1.205 1.205 1.205		EXIST.
		2 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		R/W2060
				2050
		$C + \alpha + i + \alpha + \alpha$	Ctation 1AD.	$\Delta \Delta \Delta \Delta$
50		50 50) STUNUT 142* 0	JU.U.U.
				PROJECT SHEET NO. 0081-095-829 17



EATURES RE	ELATING TO C	ONSTRUCTION	REVISED	STATE	ROUTE	STATE	- SHEET NO.
SUBJECT TO RY BY THE	CHANGE AS DEPARTMENT	DEEMED					
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XIST L/A						2080	
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ллэт L/A							
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EATURES RELATING	TO CONSTRUCTION TROL OF TRAFFIC	REVISED	STATE	ROUTE	STATE PROJECT	SHEET NO.
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FEATURES RELATING TO CONSTRU EGULATION AND CONTROL OF TRA	AFFIC) STATE	ROUTE	STATE PROJECT	
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PROJECT I SURVEYED	MANAGE BY, DA	R <i>James</i> . ATE <i>Woolpe</i>	lones, L.S. (27 ert Inc., (757)	6).696-3257_(1 549-3549_(Nov	Bristol) .24,2019)							CRC)SS S	EC	TIONS						DESIGN FE OR TO REG	ATURES R	RELATING TO CO AND CONTROL (DNSTRUCTION DF TRAFFIC	REVISED	STATE -	ROUTE	STATE PROJECT		- SHEET NO.
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PROJECT MA SURVEYED E DESIGN BY <u>T</u> SUBSURFACE	NAGER <i>James</i> BY, DATE Woolpe amara Pritchard UTILITY BY,	Jones, L.S. (27 ert Inc., (757) d, P.E. (276) 6 DATE	6).696-3257_(Br 549-3549_(Nov.2 96-3249_(Bristol	istol) '4,2019) ')				
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EATURES RELATING TO C EGULATION AND CONTROL	CONSTRUCTION OF TRAFFIC	REVISED	STATE STATE ROUTE PROJECT			
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PROJECT MANAGER James Jones, L.S. (276) 696-3257_(Bristol) SURVEYED BY, DATE Woolpert Inc., (757) 549-3549 (Nov. 24, 2019) DESIGN BY Tamara Pritchard, P.E. (276) 696-3249 (Bristol)	CROSS SECTIONS	DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED STATE OR TO REGULATION AND CONTROL OF TRAFFIC STATE STATE OR TO REGULATION AND CONTROL OF TRAFFIC STATE STATE OR TO REGULATION AND CONTROL OF TRAFFIC STATE OR TO REGULATION AND CONTROL OF TRAFFIC STATE
SUBSURFACE UTILITY BY, DATE	SCALE 1 IN. = 10 FI	NECESSARY BY THE DEPARTMENT VA. 81 0081-095-829,C-501 23
THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE		3' AT
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Ι		150 PROJECT SHEET NO. 0081-095-829 23

PROJECT MA SURVEYED B DESIGN BY ZO SUBSURFACE	NAGER <u>James</u> BY, DATE Woolpe amora Pritchard UTILITY BY,	Jones, L.S. (276) art Inc., (757) 54 1, P.E. (276) 696 DATE	_696-3257_(Bri 19-3549 (Nov.24 3-3249 (Bristal)	stol) 4,2019) 				
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FEATURES RE	ELATING TO (AND CONTROL	CONSTRUCTION OF TRAFFIC	REVISED		ROUTE	STA	TE PROJECT	- SHEET NO.
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PROJECT MANAGER James Jones, L.S. (276).696-3257_(Bristol) SURVEYED BY, DATE Woolpert Inc., (757).549-3549 (Nov. 24, 2019). DESIGN BY Tamara Pritchard, P.E. (276).696-3249 (Bristol)	CROSS SECTIONS Scale 1 in. = 10 ft	DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT.
PH PLANS		VA. 81 0081-095-829, C-501 25
THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE DE CONSTRUCTION OR THE		Z* RT
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SCALE 1 IN. = 10 FT		MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT			~ ~
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DIECT MANAGER James, Janes, LS, (276) 598 EVEYED BY, DATE Woolpart Inc., 17571 549-55 SON BY Langta, Priltonard, PE, 12751 696-324 SURFACE UTILITY BY, DATE THESE AND UN TO BE OF CON ACOUIS: 2150 2140 2130 2150 2150 2150 2150 2150
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NEVED BY, DATE Wolger Lie, <i>ICSU</i> 549-549 (New 2) IGN BY Tamena Petitologic <i>PE</i> (276) 696-3249 (New 2) ISURACE UTULTY BY, DATE PH
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EATURES RELATING TO C EGULATION AND CONTROL	CONSTRUCTION OF TRAFFIC	REVISED	STATE	ROUTE	STATE PROJECT	SHEET NO.
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	 	U			/50 РРОЈЕСТ SF	IEET NO.
					0081-095-829	27

PROJECT MANAGER James Jones, L.S. (276).696-3257_(Bristol)	CPOS	< < < < < T					DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE STATE ROUTE PROJECT	SHEET NO.
SURVEYED BY, DATE waappell lic, 123/1249-2249 (IRistol) DESIGN BY Tamara Pritchard, P.E. (276) 696-3249 (Bristol)	SCAL	$S S \subseteq C $	I UN S) FT				MAY BE SUBJECT TO CHANGE AS DEEMED NECESSARY BY THE DEPARTMENT	
PH PLANS							VA. 8/ 0081-095-829,C-50/	28
TO BE USED FOR ANY TYPE								
ACQUISITION OF RIGHT OF WAY.								
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							PROJECT SHE 0081-095-829	:et no. 28

PROJECT MANAGER James Jones, L.S. (276) 696-3257_(Bristol) SURVEYED BY, DATE Woolpert Inc., (757) 549-3549 (Nov. 24, 2019) DESIGN BY Tamara Pritchard, P.E. (276) 696-3249 (Bristol)	CROSS	SECT	IONS					DESIGN FEATURES RELATING TO CONSTRUCTION REVISED STATE OR TO REGULATION AND CONTROL OF TRAFFIC MAY BE SUBJECT TO CHANGE AS DEEMED	STATE SHEET NO.
SUBSURFACE UTILITY BY, DATE								VA.	81 0081-095-829,C-501 29
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2/30				1.818	5Z2417	2117.25 2117.25	2117.10		2/30
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2/30				EL 2119.	<u>51</u> 21/9,	L 2118.5	2116.56 71 2118.5		2130
		МАТСН	MATCH	12.51	24.51	34.51' E 36.51' E			
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									PROJECT SHEET NO. 0081-095-829 29

PROJECT MA SURVEYED E DESIGN BY Z	NAGER Ja 3Y, DATE V amora Prin	umes_Jones,L.S.(276 Voolpert Inc.,(757)5 tobard, P.E.(276)69 BY_DATE	6)_696-3257_(Bristol) _ 49-3549 (Nov.24,2019 16-3249 (Bristol)				С	ROSS scale 1	SECT IN. = 10	I ONS					DESIGN F OR TO RI MAY BE NECESSA	EATURES RELATI EGULATION AND (SUBJECT TO CHA	ING TO CONSTRUC CONTROL OF TRA INGE AS DEEMED RIMENT	CTION REVISED	STATE -	STATE ROUTE PROJECT	SHEET NO.
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Attachment D

#### UPC 116156 Crash Data July 1, 2017 to June 30, 2020 Data obtained from Virginia Roads Website

OBJECTID	CRASH_YEAR CRASH_DT	CRASH_MILITARY_TM	PERSONS_INJURED	VEH_COUNT	COLLISION_TYPE	WEATHER_CONDITION	LIGHT_CONDITION	ROADWAY_SURFACE_COND	ROADWAY_ALIGNMENT	FIRST_HARMFUL_EVENT	FIRST_HARMFUL_EVENT_LOC	DISTRACTED_NOTDISTRACTED	RNS_MP
1	2020 2020/03/03	700	0 0	1	9. Fixed Object - Off Road	5. Rain	1. Dawn	2. Wet	4. Grade - Curve	1. Bank Or Ledge	2. Shoulder	No	32.21
2	2018 2018/07/15	2003	3 1	. 1	9. Fixed Object - Off Road	5. Rain	2. Daylight	2. Wet	4. Grade - Curve	11. Jersey Wall	2. Shoulder	No	32.37
3	2019 2019/10/24	952	2 0	1	9. Fixed Object - Off Road	1. No Adverse Condition (Clear/Cloudy)	2. Daylight	1. Dry	1. Straight - Level	5. Guard Rail	2. Shoulder	Yes	32.47
4	2019 2019/10/06	1830	0 0	1	9. Fixed Object - Off Road	5. Rain	2. Daylight	2. Wet	4. Grade - Curve	5. Guard Rail	3. Median	No	32.5
5	2019 2019/02/21	1200	0 0	1	9. Fixed Object - Off Road	1. No Adverse Condition (Clear/Cloudy)	2. Daylight	6. Oil/Other Fluids	4. Grade - Curve	5. Guard Rail	2. Shoulder	No	32.5
6	2020 2020/04/19	1700	0 1	. 2	2. Angle	1. No Adverse Condition (Clear/Cloudy)	2. Daylight	1. Dry	1. Straight - Level	20. Motor Vehicle In Transport	1. On Roadway	Yes	32.6
7	2018 2018/04/15	2020	0 0	1	9. Fixed Object - Off Road	5. Rain	5. Darkness - Road Not Lighted	2. Wet	4. Grade - Curve	4. Fence Or Post	2. Shoulder	No	32.67
8	2019 2019/10/07	1825	5 0	1	9. Fixed Object - Off Road	5. Rain	2. Daylight	2. Wet	4. Grade - Curve	5. Guard Rail	2. Shoulder	No	32.7
9	2019 2019/01/02	1200	0 0	2	4. Sideswipe - Same Direction	1. No Adverse Condition (Clear/Cloudy)	2. Daylight	1. Dry	3. Grade - Straight	20. Motor Vehicle In Transport	1. On Roadway	No	32.99
10	2019 2019/12/02	1848	8 1	. 2	1. Rear End	6. Snow	5. Darkness - Road Not Lighted	2. Wet	3. Grade - Straight	20. Motor Vehicle In Transport	1. On Roadway	No	33.01
11	2020 2020/01/19	1434	4 0	2	1. Rear End	1. No Adverse Condition (Clear/Cloudy)	2. Daylight	1. Dry	3. Grade - Straight	20. Motor Vehicle In Transport	1. On Roadway	Yes	33.22

## UPC 116156 Crash Data Map

July 1, 2017 to June 30, 2020 Locations obtained from Virginia Roads website.

Begin Project

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Limits of DE Request

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Attachment E

#### Stroupe, Rebekah

From:	Pritchard, P.E., Tamara <tamara.pritchard@vdot.virginia.gov></tamara.pritchard@vdot.virginia.gov>
Sent:	Tuesday, April 6, 2021 1:45 PM
То:	Stroupe, Rebekah
Subject:	Fwd: UPC 116156 DE for Stopping Sight Distance and Superelevation

FYI

------Forwarded message -------From: **Brian Holt, P.E., PTOE** <<u>brian.holt@vdot.virginia.gov</u>> Date: Tue, Apr 6, 2021 at 1:35 PM Subject: RE: UPC 116156 DE for Stopping Sight Distance and Superelevation To: Pritchard, P.E., Tamara <<u>tamara.pritchard@vdot.virginia.gov</u>> Cc: Chase Buchanan <<u>chase.buchanan@vdot.virginia.gov</u>>, Gene Holley <<u>gene.holley@vdot.virginia.gov</u>>

We went out and ball banked the two curves and they ball banked good. So we do not need any curve warning signs or advisory speeds.

I would put this in the DE to show that we investigated mitigation strategies.

#### Brian Holt, P.E., PTOE

District Traffic Engineer /Bristol District



Virginia Department of Transportation

276-696-3414

Brian.Holt@VDOT.Virginia.gov



Wed, Apr 21, 2021 at 7:08 PM

1 message

Re: UPC 116156 - DE for Superelevation Rates Brian Holt, P.E., PTOE <brian.holt@vdot.virginia.gov> To: "Buchanan, Chase" <chase. buchanan@vdot.virginia.gov> Cc: James Jones <jamesr.jones@vdot.virginia.gov>, "Tamara Pritchard, P.E." <tamara.pritchard@vdot.virginia.gov> I concur with this DE On Wed, Apr 21, 2021 at 5:03 PM Buchanan, Chase <chase.buchanan@vdot.virginia.gov> wrote: Brian, Please review the attached design exception package for superelevation on UPC 116156. If you do not have any comments, please concur with the DE. Please let me know if you have any questions. Thank you, Chase Buchanan, P.E. VDOT Senior Roadway Engineer Bristol District Location & Design Bristol District 870 Bonham Road Bristol, VA 24201 Office: 276-696-3251 | Mobile: 540-460-5437 116156 Superelevation DE Package.pdf Brian Holt, P.E., PTOE District Traffic Engineer / Bristol District Virginia Department of Transportation VDOT 276-696-3414 Brian.Holt@VDOT.Virginia.gov



# I-81 Corridor-wide Design Guide

Appendix B





# IMPROVE 81

# Appendix B: DE/DW for Shoulder Width

#### Information needs:

- Existing roadway plans showing typical sections and plan sheets
- Proposed roadway plans showing typical sections and plan sheets
- Survey of guardrail locations and paved shoulder limits
- Digital terrain model or survey elevations to determine graded shoulder limits
- Crash history of past 3 years within project limits with crashes geospatially located

#### **References:**

- VDOT Road Design Manual Appendix A-1, page A1-15
- AASHTO Green Book Chapter 8.2.4
- AASHTO Policy on Design Standards Interstate System, Table 3

#### Process:

- Establish the existing width of paved shoulder and total shoulder throughout the project limits
- Establish the proposed width of paved shoulder and total shoulder throughout the project limits
- Conduct constraints analysis identifying unique project constraints locations in context of the entire corridor
- Tabulate the locations with paved shoulder width less than the minimum allowed per AASHTO standards (Design Exception) in the Summary of Non-Standard Paved Shoulder Widths spreadsheet as shown in Exhibit B.1 for inclusion as a table in the Design Exception Narrative
- Tabulate the locations with total shoulder width less than the minimum allowed per VDOT standards (Design Waiver) in the Summary of Non-Standard Total Shoulder Widths spreadsheet as shown in Exhibit B.2 for inclusion as a table in the Design Waiver Narrative
- Review and document constraints and purpose and need of exception (e.g., overhead bridge piers or abutment, deep fills or cuts, and/or other constraints)
- Evaluate 3 years of crash data with specific focus on the areas of shoulder widths that do not meet current standards
  - If crash data suggests an existing safety problem that aligns with locations of deficiencies and where the substandard shoulder is likely a contributing factor, evaluate appropriate mitigation measures and/or corrections
    - Issues to be considered consist of:
      - Widening shoulders
      - Underdrain

B-1

# IMPROVE 81

- Effect on ditches and slopes
- Constructability and maintenance of traffic
- If crash data does not suggest an existing safety problem that aligns with locations of deficiencies or there is no discernable crash trend, consider mitigations offered by the proposed project such as widened shoulders, improved drainage infrastructure, rumble strips, etc.
  - Mitigation strategies may include:
    - Rumble strips
    - Wider edge line markings
    - Roadside barrier
    - Emergency pull off areas
- In either scenario above, use the crash data to develop a qualitative crash analysis by applying an approved CMF for the improvement (such as widening from a certain number of lanes to a new number of lanes).
- Fill out IIM Form LD-440 (Design Exception) and/or LD-448 (Design Waiver) including supporting documentation.

#### **Supporting Documentation:**

Attachment A – Existing roadway plans (Title Sheet and Plan Sheets)

Attachment B – Excerpts from AASHTO and VDOT standards as applicable to the request

Attachment C – Proposed roadway plans (Title Sheet, Typical Sections, and Plan Sheets)

Attachment D – Summary of Non-Standard Paved Shoulder Widths (as shown in Exhibit B-1) (*The table format may need to be modified slightly to fit a project specific situation, based on number of lanes and location of widening.* 

Attachment E - 3 years of crash data in tabular format and geo-spatial exhibits

Attachment F – Shoulder width safety analysis

Attachment G - Miscellaneous information in support of request

#### Notes:

A sample approved DE/DW is attached at the end of this section.

B-2

# IMPROVE 81

Figure 2: Decision Flowchart for DE/DW for Shoulder Width



VDOT



### **I-81 Corridor Improvement Plan**

## **Project Name**

Date:

## **Summary of Deficient Paved Shoulder Widths**

VDOT Project #:

**VDOT UPC #:** 

Station Range of Deficient Paved Shoulder Section			Side of Travelway (LT/RT)	Urban/Rural?	Terrain Type	Guardrail? (YES/NO)	Truck Traffic ¹ (DDHV)	AASHTO Min. Paved Shoulder Width ²	Average Exist. Paved Shoulder Width	Proposed Paved Shoulder Width
From		То						(ft)	(ft)	(ft)
100+00.00	to	110+00.00	RT	Rural	Rolling	NO	200	10	6	8

Table B-1: Paved Shoulder Width

Notes

B-4




# **I-81 Corridor Improvement Plan**

# **Project Name**

Date:

# **Summary of Deficient Total Shoulder Widths**

VDOT Project #:

**VDOT UPC #:** 

Station Range of Deficient Total Shoulder Section		Station Range of Deficient Total Shoulder Section		of way Urban/Rural? .T)	Urban/Rural?	Terrain Type	Guardrail? (YES/NO)	Truck Traffic ¹ (DDHV)	VDOT Min. Total Shoulder Width ²	Average Exist. Total Shoulder Width (ft)	Proposed Total Shoulder Width (ft)	Notes
From		То						(ft)	(11)	(11)		
100+00.00	to	110+00.00	RT	Rural	Rolling	NO	200	12	8	10		

Table B-2: Total Shoulder Width

B-5



# VIRGINIA DEPARTMENT OF TRANSPORTATION LOCATION AND DESIGN/STRUCTURE & BRIDGE DESIGN EXCEPTION REQUEST

Design Exception Number:

(For use by NOVA Only)

Date: 9	Date: 9/16/2020								
То:	Butch Heishma	an, P.E.		Assistant State Location and Design Engineer					
From:	Alex Price, P.E.			District Location and Design Engineer					
Subject	DESIGN EXCEP	TION REQUEST							
Project Information									
UPC		116203	State Project Number		0081-080-946				
Federal Project Number		NHPP-0812(323)	District		Salem				
City/Cou	inty	Roanoke County/City of Salem	Click to choose an item.						
Start Loc	ation (From)	0.0189 Mi. North Rt. 641							
End Loca	ition (To)	0.349 Mi. North Rt. 419							
Project D	Description	I-81 Widening MM 136.6 to	I-81 Widening MM 136.6 to 141.8						
Funding	Source	Federal							

Desi	Design Exception Request For											
	Design Speed		Horizontal	Curve R	adius		Design Loa	ading Structural Capacity*				
	Lane Width		Cross Slope	e			Vertical Cl	Vertical Clearance*				
$\boxtimes$	Shoulder Width		Sight Dista	ance (Sto	pping)		Supereleva	ation Rate				
	Maximum Grade		Other									
* These are typically requested by the Bridge designer.												
Road	d and Traffic Infor	matio	on									
Curr	ent ADT	27,2	96 NB / 23,1	.49 SB	Design	ADT		31,882 NB / 27,038 S	SB			
% Tr	ucks	28.2	28.2% Design		Speed		65 MPH	Posted Speed	60 MPH			
Redu Spee	iced Design d (if applicable)	N/A			Is Project on NHS?			Yes				
Func Class	tional sification	GS-II	NT INTERST	ATE								

LD-440
(5-22-20)

Min. AASHTO Standard	10'		AASHTO Reference	A Policy on Design Standards – Interstate System - Cross Section Elements – Table 3				
Existing Dimensions	Varies 2' – 12' in attached na	' (See Table 1 arrative)	Requested Dimensions	Varies 4' to 10' (See Table 1 in attached narrative)				
Total estimated constru	ction cost of pr	oject \$300	ect \$ 300 Million					
(Based on approval of the or replacing structures,	nis exception) acquiring addit	Cost should inc ional right of w	lude item such as additional g ay, wetland mitigation, etc.	rading or paving, widening				
Additional cost to meet standard	minimum AASH	ITO \$ 26 -	35 Million (See attached narra	ative)				
Background description	on of project:	(Attach Sepa	rate Document)					
(Include a description of the general characteristics of the existing highway focusing on the features releva the proposed exception. Provide a brief description of the adjacent highway segments, highlighting existin nonstandard features when relevant to the proposed exception as well as the title sheet, typical section sh applicable plan sheets, profiles and cross sections.)								
Design Exception Deta	ails							
Purpose and need for ex standards cannot be me any future plan to upgra exception to standard)	cception; why et (include ade this	See attached narrative.						
Are there any plans to ir	nprove the app	roach roadway	oach roadway within the next ten (10) years? No					
Accident history for the number of crashes, seve crashes, types of crashe the road, rear-ends, side head-on, etc.), cause of affect the design except on types and number of a comparison of the sta average	past 3 years, erity of s (run- off- eswipes, crashes, the ion will have crashes and tewide	See attached	narrative.					
Effect of design exception	on on safety	See attached	See attached narrative.					
Mitigation of the substa element(s)	ndard design	See attached	e attached narrative.					
Has the Responsible Dis Engineer reviewed the s design and do they agre proposed mitigation me not, please explain.	trict Traffic ubstandard we with the basures? If	District Traffic Engineer has reviewed and concurs with design exception intent.						



Prepared By: Tyler Gill, P.E. - RS&H, Inc. Date: 9/16/2020 Consultant (P.E.'s Name and Company Name)

Note: The responsible person that prepares the request shall also electronically seal and digitally sign in the block above. **All signatures below shall be digital signatures**.

Remarks:

Recommended For Submission to C.O. By							
	District Location and Design Engineer						
	Or						
	District Structure and Bridge Engineer						

Recommended for	Approva	I Ву					
		Assistant State Location and Design Engineer					
		Ur					
	-	Assistant State Structure and Bridge Engineer					
		Responsible District Traffic Engineer					
		(For Crossovers Only)					
VDOT and FHWA A	pproval						
		State Location and Design Engineer					
VDOT Approval By		Or					
		State Structure and Bridge Engineer					
FHWA Approval By							

Cc: Project Manager Design Engineer State Geometric Design Engineer

# Background/Project Description

UPC 116203 proposes to widen NB and SB I-81 by a single lane in each direction between mile marker 136.6 and 141.8, approximately 5.2 miles. This project begins about 3,000 feet south of the I-81/Rt. 112 (Wildwood Rd.) interchange (Exit 137) near the I-81 over Rt. 641 (Texas Hollow Rd.) bridges and ends about 1,800' north of the I-81/Rt. 419 (North Electric Rd.) interchange. The project is located within Roanoke County and the City of Salem, as the corporate limits straddle I-81 through this portion of the corridor.



Three interchanges are found within the project: (1) Exit 137 – I-81/Rt. 112 – Partial Cloverleaf/Folded Diamond, (2) Exit 140 – I-81/Rt. 311 – Partial Cloverleaf/Folded Diamond, and (3) Exit 141 – I-81/Rt. 419 – Partial Cloverleaf. In addition to the interchanges, there are three underpasses (Rt. 635 – Goodwin Ave., Rt. 619 – Wildwood Rd./Academy St., Rt. 630 – Kessler Mill Rd.) and one overpass (Rt. 705 – Red Ln.). Included in UPC 116203 is the replacement of the bridge structures at Rt. 112 (Exit 137), Rt. 635, and Rt. 619. The Rt. 311 (Exit 140) bridges will be widened to accommodate the additional thru lane. No proposed widening or replacement work will occur on the Rt. 705, Rt. 630, or Rt. 419 (Exit 141) bridges.

The proposed widening scheme generally follows constructing an additional lane to the outside in the SB direction and to the inside in the NB direction. The typical section is mainly comprised of three 12' lanes and two 12' total shoulders per direction.

UPC 116203 was identified as Project ID #39A and 39B in the 2018 'I-81 Corridor Improvement Plan, with the goal to provide additional capacity, reduce congestion, and improve safety.

Immediately to the north of this project another, UPC 108906, is currently under construction to widen and connects the auxiliary lanes together between Exits 141 and 143. The proposed design for UPC 116203 will tie into UPC 108906 at the northern terminus.

Interstate 81 between MM 136.6 and 141.8 is classified as Urban Interstate System (GS-INT) with a design speed of 65 MPH and a posted of 60 MPH.

This design exception seeks a reduction in the required paved shoulder width of 10', as presented in the 2016 AASHTO *A Policy on Design Standards - Interstate System* Table 3, for two general locations within the project corridor. For reference the AASHTO table is shown in Figure 1. Specifically, Locations (1) and (2) are associated with the SB I-81 lane shift/lane drop/Exit 137 and Locations (3) to (6) are due to the existing Rt. 419 overpass crossing. Individual station ranges of each are identified in Table 1 below. Associated project plan sheets are provided in Appendix A.

Multiple alternatives/concepts were explored during initial project development. These centered around the location of interstate widening (inside/outside). A detailed discussion and preferred widening strategy recommendation is included in Appendix G.

Shoulders

through lanes sha Table 3. Minimu	Ill be in accordance Im Paved Shoulde	with the followir <b>r Widths</b>	ng table:		
One-Direction- al No. Through lanes	Terrain	Left Shoulder (ft)	Right Shoulder (ft)	Left Shoulder (m)	Right Shoulder (m)
2-lane	Level or Rolling	4	10	1.2	3.0
3-lane or more	Level or Rolling	10	10	3.0	3.0
2 or 3-lane	Mountainous	4	8	1.2	2.4
4-lane or more	Mountainous	8	8	2.4	2.4

#### Figure 1: Table 3 from the AASHTO Policy on Design Standards – Interstate System

Minimum paved shoulder widths in each direction of travel as a function of terrain and the number of

Where truck traffic exceeds 250 DDHV, additional shoulder width may be beneficial. Refer to AASHTO's *Green Book* for more information. Additional guidance on shoulder widths for tunnels and long bridges [overall length over 200 ft (60 m)] is provided later in this document.

Location	Proposed Paved Shoulder Width	Existing Shoulder Width	Notes
(1) NB I-81 sta. 137+00 to 152+00 - Median	Varies 10' to 6.15'	Varies 2' to 4' (2' -4' paved)	SB lane shift/drop
(2) SB I-81 sta. 536+50 to 552+00 - Median	Varies 10' to 6.15'	Varies 5'-6' (2'-3' paved)	SB lane shift/drop
(3) NB I-81 sta. 349+00 to 361+00 - Median	Varies 10' to 6.4'	Varies 5'-9' (4'-8' paved)	Rt. 419 (N. Electric Rd.)
(4) NB I-81 sta. 355+00 to 359+50 - Outside	Varies 10' to 6.58'	Approx. 12' (11' paved)	Rt. 419 (N. Electric Rd.)
(5) SB I-81 sta. 748+00 to 760+50 - Median	Varies 10' to 4.04'	Varies 5'-7' (4'-5' paved)	Rt. 419 (N. Electric Rd.)
(6) SB I-81 sta. 756+50 to 760+00 - Outside	Varies 10' to 7.8'	Approx. 12' (11' paved)	Rt. 419 (N. Electric Rd.)

Table 1	Total	Paved	Shoulder	Width	Locations	<10'
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## General Existing Roadway Characteristics

The existing characteristics of I-81 through the project limits are individual horizontal and vertically bifurcated alignments consisting of two 12' travel lanes, 2'-4' paved/4'-6' total inside shoulders, and 10'-12' paved/12'-14' total outside shoulders. Median widths (travel lane to travel lane) vary throughout the project, with the widest median of approximately 100' located near the southern project terminus and a typical median width between 40' and 60'.

Pertaining specifically to this design exception, in the area of the SB I-81 lane shift/lane drop/Exit 137, the existing NB I-81 inside shoulders vary 2'-4' with the majority paved. The SB I-81 inside shoulders have double sided guardrail and average 5'-6' to the face of rail of which 2'-3' is paved.

At the Rt. 419 overpass, the NB I-81 inside shoulder widens from 5' to 9' with guardrail present near the bridge. The paved portion of this shoulder varies from 4' to 8' over the same distance. The NB and SB I-81 outside shoulder generally maintains an 11' paved section with 12' to the face of existing roadside barrier. In the SB direction the inside shoulder is adjacent to guardrail the entire length with a 5'-7' offset to the face of barrier and a paved distance of 4'-5'. The existing deceleration lane taper for the NB I-81 Exit 141 to Rt. 419 Electric Rd. loop begins immediately north of the Rt. 419 overpass.

# Purpose and Need for Exception

The established design criteria cannot be met at the locations in Table 1 due to various existing horizontal and vertical constraints. Each location is detailed below.

#### Locations (1) & (2)

The proposed NB and SB paved inside shoulder width through this area is below standard, varying from 10' to a minimum of 6.15', to accommodate the added thru lane on I-81 without modifying the interchange at Exit 137. This location has a reduced existing median width, including existing substandard inside shoulder widths of 2'-4' (NB) and 5'-6' (SB), and is constrained due to terminating the added southbound lane prior to the Texas Hollow Rd. bridge. To provide full width shoulders, SB I-81 widening needs to occur to the outside through the Rt. 112 interchange. However, a severely

substandard existing deceleration lane to Rt. 112 requires lengthening as well, meaning 24' of additional width is needed on the new bridge. Accomplishing all widening to the outside will result in major impacts to the existing interchange configuration and require a modification. The proposed design balances the roadway impacts at this location by providing the new thru lane to the inside (median) and the auxiliary lane to the outside while minimizing the construction footprint at the interchange.

To reduce construction impacts to Exit 137, the proposed design achieves the additional thru lane widening to the inside by incorporating a horizontal lane shift through a section of tangent north of the interchange. The location of the horizontal lane shift provides an optimal solution for reducing associated bridge, interchange, geotechnical, and constructability impacts.

Accomplishing the shift north of the SB I-81 over Rt. 112 bridge decreases the construction footprint at Exit 137, specifically that of the SB loop ramp, which would require major reconstruction should the deceleration lane be an additional 12' west. The existing reduced horizontal geometrics of this loop currently contribute to a high volume of crashes at this location. Any further widening to the west would require interchange reconfiguration or aggressive geometry barely meeting standard (or requiring additional design waivers/exceptions) to tie, the former being out of the scope of this project and the latter being suboptimal.

Additionally, other realized aspects of the proposed design include: improved shoulder widths over existing; reduced need to continue to raise the SB I-81 vertical alignment as outside widening at Exit 137 requires further elevation increase to achieve a 16'6" vertical clearance; balanced/maximized (similar) inside shoulder widths between NB and SB I-81; reduced cut and potential need for retaining structures or large right of way acquisition adjacent to Skyview Rd. (frontage road); reduced/eliminated large impacts to the acid producing soils that are present in the existing cut slope between Exit 137 and the Texas Hollow Rd. bridges; maintaining the crowned roadway section improving constructability at all points south of the shift; and future accommodation of an inside/inside widening concept moving south on the I-81 corridor.

A more detailed summary of the SB I-81 lane shift is captured in Appendix H.

#### Locations (3), (4), (5), & (6)

The Rt. 419 bridge has a condition rating of fair, provides vertical clearances over I-81 greater than 16' 6", and according to the most recent inspection report does not have evidence of strikes/hits. Therefore, replacement or reconstruction of the Rt. 419 overpass is not considered a high priority element and thus outside the scope of this project. However, the existing span arrangement does not adequately accommodate three 12' thru lanes, one 12' deceleration lane, and two 10' paved shoulders (inside and outside) in each direction. This constraint results in reduced inside and outside paved shoulders at this location. Specifically, the NB inside and outside paved shoulders will vary from 10' to 6.4' and 10' to 6.58' and the SB inside and outside paved shoulders will vary from 10' to 7.8', respectively. Appendix E contains a portion of the project scope document referencing intent to leave the Rt. 419 structure in its existing location. Further, Appendix F documents the decisions/design direction agreed to between VDOT and FHWA at the project Bridge Strategy Determination Discussion. Among the items are leaving the Rt. 419 bridge in its existing configuration. The Rt. 419 overpass pier locations and the existing vertical clearance restricts small to moderate profile and cross slope adjustments, the proposed NB and SB horizontal alignments were located to maximize and balance the paved shoulder widths on either side of I-81 to the extent practical.

Another feature resulting in reduced paved shoulder width is the extension of the NB deceleration lane for the loop ramp to Rt. 419 to meet standard. Currently, this deceleration lane is less than half the AASHTO required length of 500' and does not begin until just north of the bridge. Providing additional auxiliary lane length will improve operations at this loop and increase safety.

In the SB direction, the proposed design ties directly to the southern terminus of UPC 108906. Due to its proximity to the Rt. 419 overpass, this tie condition does not allow room for a meaningful horizontal alignment shift thus the resultant reduced outside paved shoulder.

Ultimately, providing standard inside and outside shoulders along I-81 would necessitate bridge replacement of the Rt. 419 overpass. This effort was explicitly restricted from this project.

### Crash History

VDOT provided crash data for the project corridor using the most recent three year period. A total of 326 crashes occurred within the project limits, of which fourteen (4.3% of the total) took place within the Table 1 areas. There were no fatalities and three injuries associated with those crashes. Of the collisions; six were rear-end, three fixed object, three were sides wipe, one angle, and one non-collision. The three fixed object and one non-collision crash all were run of the road crashes, with two occurring in both Location (1) and (3). Location (1) and (3) each had one crash that occurred on the inside shoulder and one that impacted both. No crashes occurred in Location (4) or (6). The individual crash data and a summary table are attached as Appendix B for reference.

Within the project, not just the locations in Table 1, the NB I-81 crash rate is 30% higher than the statewide and 60% higher than the district averages. The SB I-81 crash rate is higher by 8% and 35% when compared to the statewide and district averages, respectively.

## Effects of Design Exception on Safety

Even with the reduced paved shoulder width design exception locations, the proposed roadway improvements are expected to provide a safer condition over existing. The addition of a third travel lane in each direction through the project will provide higher capacity, thus reducing congestion which is a major contributor to the volume of crashes in the corridor. In addition, there are many cases where the proposed paved shoulder width is greater than the existing.

At Locations (1) and (2) the proposed total and paved shoulder widths will exceed the existing widths for the entirety of the substandard section. This wider shoulder is expected to increase safety over the existing conditions.

A review of the existing crash history at Locations (3) to (6) does not indicate a wider shoulder would reduce crash frequency.

To study the effects of the proposed roadway improvements on crash rates (specifically the inside shoulder width), a safety analysis was performed using the crash data provided by VDOT and crash modification factors. Findings from this analysis determined the following safety benefits:

- Crashes are expected to be reduced by 30% when widening from 4 lanes to 6 lanes (CMF = 0.7)
- Fatality/injury crashes (KABC) would reduce by 13% when the inside shoulder is widened from 4' to 12' (CMF = 0.87) and 4% when widened from 4' to 6' (CMF = 0.96)
- Property Damage crashes (PDO) would reduce by 12% when widening the inside shoulder from 4' to 12' (CMF = 0.88) and 3% when widening from 4' to 6' (CMF = 0.97)

This analysis is located in Appendix C for reference.

## Mitigation of the Substandard Design Elements

FHWA's Technical Report on Mitigation Strategies for Design Exceptions suggests several measures for mitigating reduced shoulder width. Among these are rumble strips, wider pavement markings, paved shoulders, and roadside barrier.

The proposed design incorporates shoulder rumble strips and 6" edge line markings through the entire corridor, which the FHWA's document says encourages the driver's ability to stay within the travel lane both visually and audibly. In addition, at all 6 reduced paved width shoulder locations full depth paved shoulder is proposed to the face of the adjacent roadside barrier. This maximizes the paved width to the extent practical for the traveling public. Further, proper roadside barrier (guardrail, concrete median barrier, or bridge pier protection system) is provided to adequately shield vehicles from roadside hazards.

Each location is the result of a localized constraint that requires a reduction in paved shoulder width and not a consistent corridor wide approach. The total proposed reduced paved shoulder length on I-81 is roughly 6,300', this represents approximately 6% of the total shoulder length (inside and outside) of the entire project. The remaining 94% of the project contains paved shoulder widths in excess of 10'.

## Additional Cost to Meet Minimum AASHTO Standard

Due to available funding being constrained, project cost increases have severe consequences to overall project viability. Given current project cost estimates are very near the allocated SYIP funds, further cost increases jeopardize the project. The current construction estimate for this project is near \$300 million.

The estimated construction costs for a non-exception condition for each location are shown below.

#### Locations (1) & (2)

To provide full width total shoulders in this area requires moving the lane shift south of the I-81 over Rt. 112 bridges. Additional construction elements include pavement widening, raising SB I-81 and the SB bridge structure, lengthening barrier wall along SB I-81 inside shoulder, extending ramp/loop roadway improvements, culvert extensions, added retaining structure adjacent to Skyview Rd., and increased cut/fill slope impacts. The construction cost is estimated to increase approximately \$9-13 million to include these design changes.

#### Locations (3), (4), (5), & (6)

Increasing the paved shoulder width at the Rt. 419 interchange to meet VDOT standard involves replacement of the Rt. 419 over I-81 bridge, I-81 roadway widening, loop/ramp impacts, and roadway

enhancements to Rt. 419. The construction cost is estimated to increase approximately \$17 - 22 million to include these design changes.

In aggregate, the estimated total cost to meet the GS-INT standard is **\$26 - 35 million**, approximately a 9% increase over the current project construction estimate. The design fee associated with these additional improvements for UPC 116203 could add \$2-3 million to the overall project cost. Associated right of way, utility, and owner cost impacts are not included in this estimate.

# Appendices

Included as appendices to this design exception are:

- Appendix A: RFP Plan Sheets (including title sheet and typical sections) with Design Exception locations denoted
- Appendix B: Crash Data within DE Areas (Excludes Collisions with Deer)
- Appendix C: Inside Shoulder Width Safety Analysis
- Appendix D: LD-104 I-81 Traffic Data for Scoping Report and Design Purposes
- Appendix E: Excerpt from project scoping document communicating desire to not impact the Rt. 705 and Rt. 419 overpasses
- Appendix F: Bridge Strategy Memo
- Appendix G: Concept Summary Recommendation Memo
- Appendix H: I-81 Southbound Lane Shift Memo

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Appendix A





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	X241	116203		144.00	0.027				BRIDGE	SB I-81 OVER ROUTE 311
	X241	116203		144.00	0.027				BRIDGE	NB I-81 OVER ROUTE 311
	X241	116203		73.68	0.014				BRIDGE	ROUTE 419 OVER I-81
	X241	116203		33.25	0.006				BRIDGE	ROUTE 705 OVER I-81
	X241	116203		135.42	0.026				BRIDGE	NB I-81 OVER ROUTE 112
	X241	116203		85.75	0.016				BRIDGE	NB I-81 OVER ROUTE 635
	X241	116203		85.75	0.016				BRIDGE	SB I-81 OVER ROUTE 635
	X241	116203		60.41	0.011				BRIDGE	SB I-81 OVER ROUTE 619
	X241	116203		60.58	0.011				BRIDGE	NB I-81 OVER ROUTE 619
	X241	116203		135.42	0.026				BRIDGE	SB I-81 OVER ROUTE 112
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198+09.71

620+17.51

597+67.53 598+53.28

220+88.39 221+48.97

530+50.80 531+86.22

198+95.46

620+77.92

Plotted By: GillT LIMITED ACCESS HIGHWAY By Resolution of Highway Commission dated Oct. 4, 1956

dll6203001.dgn

2,000' 4,000' 0' Copyright 2020, Commonwealth of Virginia PROJECT SHEET NO.

0081-080-946

SCALE



# Appendix A

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LIMITED ACCESS HIGHWAY By Resolution of Highway Commission dated Oct. 4, 1956



# Appendix A

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THESE PLANS ARE UNFINISHED RFP PLANS AND UNAPPROVED AND ARE NOT For Information TO BE USED FOR ANY TYPE Only OF CONSTRUCTION OR THE ACOUISITION OF RIGHT OF WAY. DATE: 09/17/20 ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT- OF-WAY SHOWN ON THESE PLANS. SHEET NO. PROJECT NOT TO SCALE 2A(2) 0081-080-946





Project Location Roanoke County / City of Salem North Arrow & Scale Legend Proposed Full Depth Pavement Proposed Planing and Resurfacing/Build-Up Demolition of Pavement Low Maintenance Median Treatment Proposed Limits of Construction - Cut Proposed Limits of Construction - Fill Potential Location of Stormwater Facility ——— —— Existing Right of Way ----- Existing Easements Proposed Right of Way ---- Proposed Permanent Drainage Easement ----- Proposed Temporary Construction Easement Property Line Proposed Concrete Median Barrier Proposed Curbing Proposed Guardrail Proposed Retaining Wall REFERENCES ( PROFILES, DET AIL & DRAINAGE DESCRIPTION SHEETS, ETC.) Utility Owners IF(2) Construction Alignment Data IG(1) - IG(9) Profile Rolls 4A - 4B Structural Obstruction Zone 4S0Z RFP PLANS For Information Only DATE: 09/17/20 THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY. ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT- OF-WAY SHOWN ON THESE PLANS. **Contact Information** Craig Moore. PE Project Manager / Assistant District L&D Engineer Virginia Department of Transportation 731 Harrison Avenue Salem, VA 24153 Comments may also be sent to: I81-MM136-141@vdot.virginia.gov I-81 Widening MM 136.6 to 141.8 Roanoke County / City of Salem, Virginia 0081-080-946, P101, R201, C501 UPC 116203 Federal Project Number



# I-81 WIDENING MM 136.6 TO 141.8

# ION RT 112 I OOP STA.1208+52.73 RT 112 LPD CONST. B.L. 1204 BEGIN CONSTRUCTION STA. 1310,00.00 RT 112 RPD CONST. B.L.

N 71° 45′ 34" E NB /181 CONST.B 

R = 1,000,00' PC = 1000+00,00 PCC = 1001+84,10 DS = 45 MPH E = 6.80%

RT 112 RAMP B CURVE DATA

Curve CI PI = 1103•34J2 DELTA = 26°44′28,05" (RT)

 $D = 10^{\circ} 25' 03''$  T = 130.73' L = 256.70' R = 550.00'  $PC = 1102 \cdot 03.39$   $PT = 1104 \cdot 60.09$ 

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TCHELL

Curve TG4 PI = 14I*27,66 DELTA = 1° 57′ 23,12" (RT)

*- 0° 07′ 38*[•] - 768.36'

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PC = 133.59.30 PT = 148.95.87 DS = 65 MPH

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VERNON D. HARRIS Instr. * 180001626 0.635 AC. Tax Map * 130-1-3

RT 112 RAMP A CURVE DATA Curve TG47 PI = 1000•92.31 DELTA = 10° 32′ 54.31″ (RT) D = 5° 43′ 46″ T = 92.31′ L = 184.10′ Curve TG48 PI = 1003+06**.**75 DELTA = 3° 57′ 26.48" (RT)

*• 1° 36′ 50*″

R = 3,550,00' PCC = 1001+84,10 PT = 1004+29,30 DS = 45 MPH E = 2,80%

Curve RTEII2I PI = I409•40.39 DELTA = I8° 37′ 27.30° (RT) D = 0° 59′ 57° T = 940.39′ L = I.864.19′ R = 5.735.00′ PC = I400•00.00 PRC =I418•64.19

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Curve TG49 PI = 1011•64.42 DELTA = 14° 39′ 45.33° (LT) D = 6°01′52″ T = 122.23′ L = 243.11′ R = 950.00′ PC = 1010•42.19 PT = 1012•85.31

Curve C3 PI = I20I+28.6I DELTA = 54° 26′ 40.24" (RT) D = 22° 55′ 06" T = I28.6I′ L = 237.56' R = 250.00' PC = 1200.00.00 PCC = 1202.37.56 DS = 30 MPH E = 8.00%

Instr. L. SUTPHIN Tax Map AC.

RT 112 RAMP D CURVE DATA

ST A. 1005+10.00

Curve C6 PI = 1304+18.43 DELTA = 55° 55′ 52.92" (LT) D = 19° 25′ 20" T = 156.63′ L = 287.98′ R = 295.00′ PC = 1302+61.80 PT = 1305+49.78











- END CONSTRUCTION ST A. 1203+10.00 RT II2 LPD CONST. B.L.

# Appendix A







Project Location Roanoke County / City of Salem North Arrow & Scale X Legend Proposed Full Depth Pavement Proposed Planing and Resurfacing/Build-Up Demolition of Pavement Low Maintenance Median Treatment | [C Proposed Limits of Construction - Cut Proposed Limits of Construction - Fill Potential Location of Stormwater Facility ——— —— Existing Right of Way ----- Existing Easements Proposed Right of Way ----- Proposed Permanent Drainage Easement ----- Proposed Temporary Construction Easement ——[₽]——— Property Line Proposed Concrete Median Barrier Proposed Curbing Proposed Guardrail Proposed Retaining Wall REFERENCES (PROFILES,DETAIL & DRAINAGE DESCRIPTION SHEETS, ETC. IF(2) Utility Owners IG(1) - IG(9) Construction Alignment Data Profile Rolls RFP PLANS For Information Only DATE: 09/17/20 THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY. ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT- OF-WAY SHOWN ON THESE PLANS. Contact Information Craig Moore. PE Project Manager / Assistant District L&D Engineer Virginia Department of Transportation 731 Harrison Avenue Salem, VA 24153 Comments may also be sent to: I81-MM136-141@vdot.virginia.gov I-81 Widening MM 136.6 to 141.8 Roanoke County / City of Salem, Virginia 0081-080-946, P101, R201, C501 UPC 116203 Federal Project Number





# I-81 WIDENING MM 136.6 TO 141.8

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VDDT Virginia Department of Transportation



Project Location	
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Proposed Planing and Resurfacing/Build-Up	
Demolition of Pavement	
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ACQUISITION OF RIGHT OF WAY.	PC = 329·15.51       PC = 354·82.43       PCC = 361·29.74       PCC = 73         PT = 349·68.25       PCC = 361·29.74       PT = 369·65.91       PT = 75         DS = 65 MPH
RELOCATIONS MAY BE REQUIRED	L = 2.00%         L = 2.40%         L = 5.00           RT 419 LOOP B CURVE DATA            Curve C28         Curve C29         Curve C30         Curve C30
WAY SHOWN ON THESE PLANS.	PI = 300I*04JI       PI = 3007*06.79       PI = 3007*52.83       PI = 3105         DELTA = 43° 38' 37.64" (RT)       DELTA = 133° 13' 42.60" (RT)       DELTA = 14° 16' 49.84" (RT)       DELTA = 100 JULY         D = 22° 02' 13"       D = 26° 02' 37"       D = 16° 36' 27"       D = 21° 1.         T = 1004 JU       T = 50874'       T = 50874'       T = 43° 20'
Craig Moore. PE	Image: constraint of the state of the s
Project Manager / Assistant District L&D Engineer Virginia Department of Transportation 731 Harrison Avenue Salem, VA 24153	DS = 25 MPH DS = 25 MPH E = 6.80% E = 7.30% <u>RT 419 LOOP D CURVE DATA</u>
Comments may also be sent to: I81-MM136-141@vdot.virginia.gov	Curve C23       Curve C24       Curve C2         PI = 3204·28.9I       PI = 3209·00.60       PI = 330         DELTA = 96° 10' 33.78" (RT)       DELTA = 93° 19' 22.79" (RT)       DELTA =
I-81 Widening MM 136.6 to 141.8 Roanoke County / City of Salem, Virginia	$\begin{array}{cccccccccccccccccccccccccccccccccccc$
0081-080-946, P101, R201, C501 UPC 116203 Federal Project Number	PC = 3200*00.00 PCC = 3206*46.26 PC = 3. PCC = 3206*46.26 PT = 3210*37.17 PT = 3. DS = 25 MPH E = 5.80%

# PLAN ROLL I-81 WIDENING MM 136.6 TO 141.8

# 2055 <del>PCC3</del>206+46,26 94.26+202250 320f DO NOT DISTURB— EXISTING MARKER/ MEMORIAL 605c END CONSTRUCTION ST A. 3203+00.00 RT 419 LPD CONST. B.L. CONST RUCT 1011 — ST A. 3311+75.00

Mixed Hardwoods/Softwoods & Brush

00 CE Location (5) Shoulder Width 35°06′10″/ Varies 4.04' to 10 SB 181 CONST.B. N-35-30-34 E----- NB-181 CONST.B.L. ation (3) Location (4) der Width Shoulder Width 6.4' to 10 _____ Varies 6.58' to 10 Mixed Hardwoods/Softwoods & Brush <u>BEGIN PROJECT 0081-080-946, B681</u> ST_R A. 357+96.10 NB 181 CONST. B.L. - 1145,4C - 1145,4C 10 + 1157,1i 10 + 1152,957 Junchon Box Solar Panel On 115,35, of 110,10,12,957 Junchon Box Solar Panel On 110,13,57 of 111,10,13,57 of 1111,10,13,57 of 111,10,13,57 of 111,10,13,57 of 1111,10, Mixed Hardwoods/Softwoods R/W Mon. Netal Sign ' R/W Mon. CORDIAL HOSPITALITY LLC Instr.* PBI2-24/25 2.35 AC. Tax Map * 4-I-1.2

SHV/1 HOSPITALITY LL Mistr. * 14000916 // 4.378 AC. // Tax Map * 4-1-1 Traffic Signal Pole 800063 A Traffic Vault Traffic Signal Cabinets 800063 SB 181 CURVE DATA Curve TG27 PI = 768•56.32 DELTA = 7° 45′ I3.39° (RT) D = 0° 42′ 30° T = 548.10′ L = 1.094.53′ R = 8.088.00′ PC = 763•08.22 PCC = 774•02.75 Curve C2I PI = 3105+90.33 DELTA = 56*36' 08.85* (LT) D = 21*13' 14* T = 145.39' L = 266.73' R = 270.00' PC = 3104+44.95 PT = 3107+11.68 RT 419 RAMP D CURVE DATA Curve C22 PI = 3111+60.66 D = 14*19' 26* T = 234.86' L = 424.74' R = 400.00' PC = 3109+25.79 PT = 3113+50.53 DS = 35 MPH E = 7.90% RT 419 RAMP D CURVE DATA CORDINAT 1500 * 4.1-11 TOX Curve C26 PI = 3311+97.35 DELTA = 88° 34' 12.76" (RT) D = 13° 58' 28" T = 399.89' L = 633.80' R = 410.00' PC = 3307+97.46 PT = 3314+31.25 DS = 35 MPH E = 7.80% Curve C25 PI = 3305•55.79 DELTA = 91° 35′ I2.49° (LT) D = I7° 54′ I8° T = 328.99′ L = 5II.52′ R = 320.00′ PC = 3302•26.8I PT = 3307•38.32



# Appendix B

	I-81 Crash Summary Within Design Exception Areas										
DE Location	Station ⁽¹⁾	Mile Marker	Direction	Vehicle 1 ^(a)	Vehicle 2 ^(b)	Crash Location ^(e)	Crash ID	Type of Collision	Injury		
	139+50	137.4	NB	Car ^(c)	Car ^(c)	Inside Lane/Shoulder	15861977	Sideswipe - Same Direction	None		
	142+00	137.4	NB	Car ^(c)	Car ^(c)	Inside Lane	15547702	Angle	None		
1	149+00	137.5	NB	Car ^(c)	-	Inside Shoulder	15114196	Fixed Object - Off Road	Minor		
1	149+00	137.5	NB	Car ^(c)	Car ^(c)	Inside Lane	15473621	Rear End	None		
	151+00	137.5	NB	Car ^(c)	Car ^(c)	Inside Lane/Shoulder	15576327	Rear End	None		
	153+00	137.6	NB	Heavy Truck ^(d)	-	Both	15153806	Non-Collision - Overturned	Minor		
2	538+50	137.3	SB	Heavy Truck ^(d)	Heavy Truck ^(d)	Inside Lane/Outside Shoulder	15436048	Rear End	None		
2	541+00	137.4	SB	Car ^(c)	Heavy Truck ^(d)	Inside Lane/Shoulder	15622911	Sideswipe - Same Direction	None		
	350+00	141.3	NB	Car ^(c)	Car ^(c)	Inside Lane	15851709	Sideswipe - Same Direction	None		
	354+50	141.4	NB	Car ^(c)	Car ^(c)	Inside Lane/Shoulder	15031606	Rear End	None		
3	355+00	141.4	NB	Car ^(c)	-	Both	15304083	Fixed Object - Off Road	Minor		
	355+50	141.4	NB	Car ^(c)	Car ^(c)	Inside Lane	15432784	Rear End	None		
	356+50	141.4	NB	Car ^(c)	-	Inside Lane/Shoulder	15850924	Fixed Object - Off Road	None		
5	747+50	141.3	SB	Car ^(c)	Car ^(c)	Inside Lane	15065990	Rear End	None		

 Notes

 (1) All stations are approximated based on the GPS coordinates provided in the crash report.

 (2) Any additional vehicles involved in the accident are not included in this table.

 (3) Collisions with deer not included within this summary.

 Definitions:

 (1) Vehicle and the vehicle

(a) Vehicle 1 - Vehicle most at fault

(b) Vehicle 2 - Vehicle less or not at fault

(c) Car - Passenger vehicle weighing less than 10,000 lbs.

(d) Heavy Truck - Vehicle weighing more than 10,000 lbs.

(e) Crash Location - Indicates the side of roadway the vehicle crashed.

'Both' indicates vehicle ran off to once side of the road, and swerved across the travel lanes to the other side of the road.

0

0

# Crash Report





#### Crash Information

Location of First Harmful Event	3. Median	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	1. Main-Line Roadway
Light Condition	5. Darkness - Road Not Lighted	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	3. Grade - Straight	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	8. Non-Collision

#### **Crash Description**

VEHICLE 1 MADE AN ERRATIC LANE CHANGE AND WENT INTO THE MEDIAN. THE OPERATOR OVER-CORRECTED AND LOST CONTROL CAUSIN VEHICLE TO OVERTURN AND STRUCK THE GUARD RAIL.

Appendix B Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

#### 01/08/2020 12:26:56PM

# **Crash Report**

Driver Informatio	10/15/1989	Age 27	Vehicle Info	ormation 1	
	Driver's Action	40. Fail to Maintain Proper Cont	trol	Vahiela Manauvar	14 Changing Lanes
Condition of Drive	r Contributing to	1. No Defects			
Driver \	vision Obscured	1. Not Obscured		Skidding Tire / Mark	2. After Application of Brakes
Type of Dri	iver Distractions	14. No Driver Distraction		Vehicle Body Type	23. Truck - Single Unit Truck (3 Axles or More
	Drinking	6. Unknown		Volliolo Dody Type	<b>5 1</b> (1
Method of Alcoho	ol Determination	Not Applicable		Vehicle Damage	6. Totaled
	Drug Use	3. Unknown		Vehicle Condition	1. No Defects
	Driver's License	X/	Spec. Fur	nction Motor Vehicle	1. No Special Function
Commercial	Driver's License	Yes 2. Len and Shoulder Dalt		EMV in service	Not Applicable
Safety E	-quipment Used	3. Lap and Shoulder Belt		Iruck Cover	
<b>—</b>	Air Bag	2. Not Deployed	0	Vehicle Disabled	Yes
Elect	ted from venicle	1. Not Ejected	Comm	ercial Niotor Venicle	Voc
	Date of Death	3 Minor/Possible Injury		Iowed	
	injury type	5. WIIIOI/I OSSIDIC IIIJUI y		Oversized	No Underride No
	EMS Transport	Yes		Initial Impact Area	9 Left side - middle
S	ummons Issued	1. Yes		Direction of Travel	North
				Crash Events: 1	28 Ban Off Boad
				2	30. Overturn (Rollover)
Speed	d Before	Maximum ALL	Passengers Age Count	3	5. Guard Rail
C	rash Spe	ed Limit Safe Speed <	8 8-17 18-21 > 21	4.	Not Provided
	65	60 60 0	0 0 0	Most Harmful	30. Overturn (Rollover)
Weight o	over 10,000 lbs	Yes	Seats 9 or more No		Hazardous Materials Placard No
Commercial Motor	Vehicle Section				
Vehi	cle Configuration	9. Tractor/Semi-trailer (One Trail	iler)		
	Cargo Body Type	3. Van/Enclosed Box		License Class	
	GVWR/GCWR	3. Greater Than 26,000 lbs.	Col	mmercial Endorsemen	t
Hazardous Material					
Hazardous	s Material Placard	No		HM Class	
	HM 4-Digit			HM Cargo Present	No
H	IM Placard Name			HM Cargo Released	No
Carrier Identificatio	n				
Commericial Mc	otor Carrier Name	FONTANA			
	US DOT# / State				
Commercial /	Non-Commercial	1. Interstate Carrier			
Passenger Informati	ion				
	EMS Transport		S	afety Equip Used	
	Date of Death		Airbag I	Deployment Type	
Position	In / On Vehicle		Ejected fi	rom Vehicle Type	

Injury Type





#### **Crash Information**

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	1. Main-Line Roadway
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	1. Rear End

#### **Crash Description**

VEHICLE #3 WAS SLOWING FOR HEAVY TRAFFIC. VEHICLE #1 STRUCK VEHICLE #2 AND PUSHED VEHICLE #2 INTO VEHICLE #3. VEHICLE #4 1 STRUCK VEHICLE #1 IN THE REAR.

Appendix B Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

# **Crash Report**

Driver Informatio 07/08/1994	Age 22	Vehicle Information 1	
Driver's Action	12. Following Too Close	Vehicle Maneuver	1. Going Straight Ahead
Condition of Driver Contributing to	1. No Defects		A Na Visible Okid Maul/Tim Maul
Driver Vision Obscured	1. Not Obscured	Skidding Tire / Mark	4. No VISIDIE SKIG Mark/ IIre Mark
Type of Driver Distractions	1 Had Not Been Drinking	Vehicle Body Type	1. Passenger car
Method of Alcohol Determination	Not Applicable	Vehicle Damage	6. Totaled
Drug Use	2. No	Vehicle Condition	1. No Defects
Driver's License		Spec. Function Motor Vehicle	1. No Special Function
Commercial Driver's License	NotProvided	EMV in service	Not Applicable
Safety Equipment Used	3. Lap and Shoulder Belt	Truck Cover	Not Applicable
Air Bag	2. Not Deployed	Vehicle Disabled	Yes
Ejected from Vehicle	1. Not Ejected	Commercial Motor Vehicle	No
Date of Death		Towed	Yes
Injury Type	6. No Injury (driver only)	Oversized	No Cargo Spill No
EMS Transport	No	Override	12 Front
Summons Issued	1. Yes	Initial Impact Area	North
		Crash Events: 1	20 Motor Vehicle In Transport
		2	20. Motor Vehicle In Transport
Speed Before	Maximum	ALL Passengers Age Count 3.	Not Provided
Crash Spee	ed Limit Safe Speed	< 8 8-17 18-21 > 21 4.	Not Provided
60	60 40	0 0 0 0 Most Harmful	20. Motor Vehicle In Transport
Commercial Motor Vehicle Section Vehicle Configuration	Not Provided	Seals 9 of more into	nazardous Materiais Pracard
Cargo Body Type	Not Provided	License Class	
GVWR/GCWR	Not Provided	Commercial Endorsemen	t
Iazardous Material			
Hazardous Material Placard		HM Class	
HM 4-Digit		HM Cargo Present	
HM Placard Name		HM Cargo Released	
Carrier Identification			
Commericial Motor Carrier Name			
Commercial / Non Commercial	Not Provided		
Commercial / Non-Commercial	i tot i tottada		
Passenger Information			
EMS Transport		Safety Equip Used	
Date of Death		Airbag Deployment Type	
Position In / On Vehicle		Ejected from Vehicle Type	
		Injury Type	

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 5. Slowing or Stopping Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

The state of Delayer Distance time	14 No Driver Distraction	
Type of Driver Distractions	1 Had Nat Days Driveling	Vehicle Body Type 1. Passenger car
Drinking	1. Had Not Been Drinking	
Method of Alcohol Determination	Not Applicable	Vehicle Damage 8. Other
Drug Use	2. No	Vehicle Condition 1. No Defects
Driver's License		Spec. Function Motor Vehicle 1. No Special Function
Commercial Driver's License	NotProvided	EMV in service Not Applicable
Safety Equipment Used	3. Lap and Shoulder Belt	Truck Cover Not Applicable
Air Bao	2. Not Deployed	Vehicle Disabled No
Fiected from Vehicle	1 Not Ejected	Commercial Motor Vehicle No
Data of Doath	1. 1 lot Ljourda	Towad No
Date of Death	6 No Injury (driver only)	Overeited No
Injury Type	0. No mjury (uriver omy)	
EMS Transport	No	Override NO Underride NO
Summons Issued	2. No	Initial Impact Area <b>6. Rear</b>
		Direction of Travel North
		Crash Events: 1. 20. Motor Vehicle In Transport
Speed Defere	Maximuma	2. 20. Motor Vehicle In Transport
Speed Belore	Maximum	ALL Passengers Age Count 3. Not Provided
Clash Spe	ed Limit Sale Speed	< 8 8-17 18-21 > 21 4. Not Provided
40	60 40	1 1 0 1 Most Harmful 20. Motor Vehicle In Transport
Weight group 10,000 lbs	No	Caste Community No.
weight over 10,000 lbs	110	Seats 9 of more into mazardous materiais macard into
Commercial Motor Vehicle Section		
Vehicle Configuration	Not Provided	
Cargo Body Type	Not Provided	License Class
GVWR/GCWR	Not Provided	Commercial Endorsement
Hazardous Material		
Hazardous Material Placard		HM Class
Hazardous Material Placard		
HM 4-Digit		HM Cargo Present
HM Placard Name		HM Cargo Released
Carrier Identification		
Commericial Motor Carrier Name		
US DOT# / State		
Commercial / Non-Commercial	Not Provided	
Passenger Information		
EMS Transport		Safety Equip Used
Date of Death		Airbag Deployment Type
Position In / On Vehicle		Ejected from Vehicle Type
		Injury Type
Driver Informatio 12/30/1975	Age 41	Vehicle Information 3
Driver's Action	1. No Improper Action	Vahiala Manauwar, 5. Slowing or Stopping
Condition of Driver Contributing to	1. No Defects	
Driver Vision Obscured	1. Not Obscured	Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark
Type of Driver Distractions	14. No Driver Distraction	Matter Date Town O Truck Disk up/Deconder Truck
Drinking	1. Had Not Been Drinking	Vehicle Body Type 2. Truck - Pick-up/Passenger Truck
Method of Alcohol Determination	Not Applicable	Vehicle Damage 8. Other
Drug Llee	2. No	Vehicle Condition 1. No Defects
Driver's Lisense		Spec Eulertion Mater Vahiale 1 No Special Function
Driver's License		
	NotDrovidad	The Article Annual Annua
Commercial Driver's License	NotProvided	EMV in service Not Applicable
Commercial Driver's License Safety Equipment Used	NotProvided 3. Lap and Shoulder Belt	EMV in service Not Applicable Truck Cover Not Applicable

Crash300

Appendix B

Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

		Crash Report	
Ejected from Vehicle Date of Death Injury Type EMS Transport Summons Issued	1. Not Ejected 6. No Injury (driver only) No 2. No	Commercial Motor Vehicle No Towed No Oversized No Cargo Spill No Override No Underride No Initial Impact Area 6. Rear Direction of Travel North Crash Events: 1. 20. Motor Vehicle In Transport	
Speed Before Crash Spe 40	Maximum ed Limit Safe Speed 60 40	ALL Passengers Age Count       2. Not Provided         < 8       8-17       18-21       > 21         0       0       0       0         Most Harmful       20. Motor Vehicle In Transport	
Weight over 10,000 lbs	No	Seats 9 or more No Hazardous Materials Placard No	
Commercial Motor Vehicle Section Vehicle Configuration Cargo Body Type GVWR/GCWR Hazardous Material	Not Provided Not Provided Not Provided	License Class Commercial Endorsement	
Hazardous Material Placard HM 4-Digit HM Placard Name Carrier Identification		HM Class HM Cargo Present HM Cargo Released	
Commericial Motor Carrier Name US DOT# / State Commercial / Non-Commercial	Not Provided		
Passenger Information EMS Transport Date of Death Position In / On Vehicle		Safety Equip Used Airbag Deployment Type Ejected from Vehicle Type Injury Type	
Driver Informatio 11/28/1959	Age 57	Vehicle Information 4	
Driver's Action Condition of Driver Contributing to Driver Vision Obscured	<ol> <li>Following Too Close</li> <li>No Defects</li> <li>Not Obscured</li> <li>No Driver Distraction</li> </ol>	Vehicle Maneuver 1. Going Straight Ahead Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark	
Method of Alcohol Determination Drug Use	<ol> <li>No Dirver Distraction</li> <li>Had Not Been Drinking</li> <li>Not Applicable</li> <li>No</li> </ol>	Vehicle Body Type 2. Truck - Pick-up/Passenger Truck Vehicle Damage 8. Other Vehicle Condition 1. No Defects	
Commercial Driver's License Commercial Driver's License Safety Equipment Used Air Bag Ejected from Vehicle	Yes 3. Lap and Shoulder Belt 2. Not Deployed 1. Not Ejected	EMV in service Not Applicable Truck Cover Not Applicable Vehicle Disabled No Commercial Motor Vehicle No	
Date of Death Injury Type EMS Transport	6. No Injury (driver only) No	Towed No Oversized No Cargo Spill No Override No Underride No	

Summons Issued 1. Yes

Crash Events: 1. 20. Motor Vehicle In Transport

Initial Impact Area 12. Front

Direction of Travel North

Crash300

Appendix B

Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

# Crash Report

	Speed Before		Maximum	ALL Pa	assend	ers Age	Count	2.	Not Provided	
	Crash	Speed Limit	Safe Speed	< 8	8-17	18-21	> 21	3.	Not Provided	
	60	60	40	0	1	0	1	4.	Not Provided	
								Most Harmful	20. Motor Vehicle In Transpo	ort
	Weight over 10,000	lbs <b>No</b>			Seats 9	9 or mor	e No		Hazardous Materials Placard	No
Commercia	al Motor Vehicle Sect	tion								
	Vehicle Configu	ration Not Prov	ided							
	Cargo Body	Type Not Prov	ided					License Class		
	GVWR/G	CWR Not Prov	ided				Con	nmercial Endorsemen	t	
Hazardous	Material									
Н	azardous Material Pl	lacard						HM Class		
	HM 4-	-Digit						HM Cargo Present		
	HM Placard	Name						HM Cargo Released		
Carrier Ide	ntification									
Comme	ricial Motor Carrier	Name								
	US DOT#/	State								
Com	nercial / Non-Comm	ercial Not Prov	ided							
Dassanger l	nformation									
rassenger i	EMS Trong	port					Sa	faty Equip Used		
	Date of De	pon					Sa Airbag D	anicy Equip Used		
	Position In / On Veh	icle				Б	iected fr	om Vehicle Type		
		liele				L	neereu m	Injury Type		
H Carrier Ide Comme Comm Passenger I	azardous Material Pl HM 4 HM Placard 1 ntification ricial Motor Carrier 1 US DOT# / mercial / Non-Comm (information EMS Trans) Date of Do Position In / On Veh	lacard -Digit Name State ercial <b>Not Prov</b> port eath icle	ided			E	Sa Airbag D jected fro	HM Class HM Cargo Present HM Cargo Released epeloyment Type om Vehicle Type Injury Type		

_					· · ·						
	Document Number Revised Report Crash Date City / Town of Location of Crash	172335215 0 Sunday INTERSTATE No - At Inters	08/20/2017 E 81 section With or	1035 0.20 Miles \$	Jurisdiction County of Crash Landmarks at Scene Railroad Crossing ID South of ROUTE 419	Roanoke County Roanoke	Mile Marker Nun	GPS L 37.322	at. GF 2 <b>160 -8</b> Number	PS Long 30.03554 r of Vehi 2	a. 40 icles
	Crash Image		 INTE	#1  ERSTATE 81	[#1			Fatalities No Fataliti Injuries No Injuri	on-Pedest es Pedest es Pedest	trian 0 trian 0 trian 0	)

#### Crash Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	1. Main-Line Roadway
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	6. Traffic Lanes Marked	Work Zone Workers Present	Not Provided
Roadway Alignment	7. Dip - Straight	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	1. Rear End

#### **Crash Description**

VEHICLE #2 WAS STOPPED IN THE TRAVEL LANE DUE TO ANOTHER MOTOR VEHICLE ACCIDENT. VEHICLE #1 THEN RAN INTO THE BACK

OF VEHICLE #2. VEHICLE #1, FRONT END DAMAGE. VEHICLE #2, DAMAGE TO THE RIGHT REAR.

Appendix B Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

# **Crash Report**

Driver Informat	iol 04/13/1970	Age 47				Vel	nicle Info	ormation	1			
Condition of Dr	Driver's Actic	n 12. Follo	wing Too Close fects					Vehicle	Vaneuver	5. Slowing or Sto	opping	
Drive	er Vision Obscure	d 1. Not O	bscured		Skidding Tire / Mark					4. No Visible Ski	d Mark/Tire Ma	ark
Type of	Driver Distraction Drinkin	s 14. No E g 1. Had N	river Distraction ot Been Drinking					Vehicle E	Body Type	3. Van		
Method of Alc	Method of Alcohol Determination Not Applicable			Vehicle Damage					8. Other			
	Drug Us	e 2. No						Vehicle	Condition	1. No Defects		
	Driver's Licens	е				S	bec. Fur	nction Mot	or Vehicle	1. No Special Fu	Inction	
Commerc	ial Driver's Licens	e NotProvi	ded					EMV	in service	Not Applicable		
Safety Equipment Used 3. Lap and Shoulder Belt							Tru	uck Cover	Not Applicable			
Air Bag 2. Not Deployed		eployed					Vehicle	Disabled	Yes			
E	ected from Vehic	e 1. Not Ej	ected				Comm	ercial Mot	or Vehicle	No		
	Date of Deat	h							Towed	Yes		
	Injury Typ	e 6. No Inj	ury (driver only)	Oversized					No	Cargo Sp	ill No	
		+ No							Override	No	Underrid	e No
	EIVIS Transpo							Initial Im	pact Area	12. Front		
	Summons issue	1. ICS						Directior	of Travel	South		
								Crash	Events: 1.	20. Motor Vehicl	e In Transport	
0.0	and Defens		Marinerum				Count	7	2.	Not Provided		
Sp	Crash Ou		Iviaximum Safa Spood	ALL Pa	asseng	ers Age	Count		3.	Not Provided		
	40	eed Limit		< 8 2	8-17	18-21	> 21		4.	Not Provided		
	40	60	0	2	0	0	0	Mos	st Harmful	20. Motor Vehicl	e In Transport	
Weig	ht over 10,000 lbs	No			Seats	9 or moi	re No			Hazardous Materi	als Placard	No

#### Commercial Motor Vehicle Section

Vehicle Configuration Not Provided Cargo Body Type Not Provided GVWR/GCWR Not Provided

#### Hazardous Material

Hazardous Material Placard HM 4-Digit

HM Placard Name

#### Carrier Identification

Commercial Motor Carrier Name US DOT# / State Commercial / Non-Commercial Not Provided

#### Passenger Information

EMS Transport Date of Death Position In / On Vehicle

License Class Commercial Endorsement

> HM Class HM Cargo Present HM Cargo Released

> > 2

Safety Equip Used Airbag Deployment Type Ejected from Vehicle Type Injury Type

Vehicle Information

Driver Informatio 07/09/1946

Age 71

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 8. Stopped in Traffic Lane

Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Ty	Type of Driver Distractions 14. No Driver Distractio							Vahiala Dady Type	22 Truck - Sport Utility Vehicle (SUV)				
	Drinki	ng 1. Had N	Not Been Drinking					venicle Body Type	22. Truck - Sport Ounty		307)		
Method of	of Alcohol Determinati	on Not App	olicable					Vehicle Damage	8. Other				
	Drug U	se 2. No						Vehicle Condition	1. No Defects				
	Driver's Licen	se				S	pec. Fun	ction Motor Vehicle	1. No Special Function	1. No Special Function			
Com	mercial Driver's Licen	se NotProv	vided					EMV in service	Not Applicable				
	Safety Equipment Us	ed 3. Lap a	nd Shoulder Belt					Truck Cover	Not Applicable				
	Air B	ag 2. Not E	Deployed					Vehicle Disabled	Yes				
	Ejected from Vehi	le 1. Not E	jected		Commercial Motor Vehicle No								
	Date of Dea	th						Towed	Yes				
	Injury Ty	oe 6. No In	jury (driver only)					Oversized	No	argo Spill	No		
EMS Transport No								Override	No	Underride	No		
	EMS Transp	ort NO						Initial Impact Area	6. Rear				
	Summons Issu	ed 2. No						Direction of Travel	West				
								Crash Events: 1.	20. Motor Vehicle In Tr	ransport			
						•	0 1	2.	Not Provided				
	Speed Before		Maximum	ALL Pa	assenge	ers Age	Count	3.	Not Provided				
				< 8	8-17	18-21	> 21	4.	Not Provided				
	U	00	0	0	0	0	0	Most Harmful	20. Motor Vehicle In Tr	ransport			
Commercia	Weight over 10,000 lbs al Motor Vehicle Section	No			Seats 9	or mo	re No		Hazardous Materials Pla	card N	0		
	Vehicle Configurati	Dn Not Pro	vided					×					
	Cargo Body Ty	pe Not Pro	vided	License Class									
· · ·	GVWR/GCV	R Not Pro	vided				Coi	nmercial Endorsemen	t				
Hazardous	Material	1											
E	lazardous Material Place	rd						HM Class					
	HM 4-Di	21t						HM Cargo Present					
с [.] Ц	HM Placard Na	ne						HM Cargo Released					
	intification												
Comme	US DOT	ne											
C	US DOT#/St	ite	vided										
Com	mercial / Non-Commerc	al not i to	viucu										
D													
Passenger	Information						a	fata E ania Maral					
	ENIS Transpor						Sa	alety Equip Used					
	D ( CD )						A Lot T	)1					
	Date of Death	l					Airbag I	Deployment Type					
	Date of Death Position In / On Vehicle	1				1	Airbag I Ejected fr	Deployment Type rom Vehicle Type					
	Date of Death Position In / On Vehicle	L ;				1	Airbag I Ejected fr	Deployment Type rom Vehicle Type Injury Type					

0

# **Crash Report**





#### Crash Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	1. Main-Line Roadway
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	9. Fixed Object - Off Road

#### **Crash Description**

DRIVER OF VEHICLE 1 LOST CONTROL OF VEHICLE CAUSING HER TO RUN OFF THE LEFT SIDE OF THE ROAD, THEN RAN OFF THE RIGHT SIDI ROAD AND STRUCK A GUARDRAIL.

Appendix B Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

#### 01/08/2020 12:42:08PM

# **Crash Report**

Driver Informatio	oj 11/14/1996	Age 20				Veł	icle Info	ormation 1		
	Driver's Action	40. Fail to M	laintain Prope	r Contro	ol			Vahiala Manauwar	10 Ran Off Road	- Left
Condition of Driv	er Contributing to	1. No Defect	S					venicie maneuver	TO: I tall Off Toad	- Lon
Driver	Vision Obscured	1. Not Obscu	ured				:	Skidding Tire / Mark	1. Before Applicati	on of Brakes
Type of D	Driver Distractions	14. No Drive	er Distraction					Vehicle Body Type	1 Passenger car	
	Drinking	1. Had Not H	Been Drinking	;				Verhole Dody Type	in acconger car	
Method of Alcol	hol Determination	Not Applical	ble					Vehicle Damage	6. Totaled	
	Drug Use	2. No						Vehicle Condition	1. No Defects	
	Driver's License					Sp	ec. Fur	nction Motor Vehicle	1. No Special Fun	ction
Commercia	al Driver's License	NotProvided						EMV in service	Not Applicable	
Safety	Equipment Used	3. Lap and S	houlder Belt					Truck Cover	Not Applicable	
	Air Bag	2. Not Deplo	oyed					Vehicle Disabled	Yes	
Eje	cted from Vehicle	1. Not Ejecte	ed				Comm	ercial Motor Vehicle	No	
	Date of Death							Towed	Yes	
	Injury Type	3. Minor/Pos	ssible Injury					Oversized	No	Cargo Spill N
	EMS Transport	Ves						Override	No	Underride N
	Summons Issued	1 Yes						Initial Impact Area	12. Front	
	ourninons issued	1. 100						Direction of Travel	North	
								Crash Events: 1.	28. Ran Off Road	
She	ed Before		Maximum		assendi	ors Ano	Count	2.	28. Ran Off Road	
oper-	Crash Sne	ed Limit S	Safe Speed	< 8	8_17	18_21	> 21	3.	5. Guard Rail	
	60	60	60	1	0	0	0	4.	Not Provided	
								Most Harmful	5. Guard Rail	
Weight	t over 10,000 lbs	No			Seats 9	9 or mor	e No		Hazardous Materials	s Placard No
Commercial Moto	v Vahiala Saatian									
Vol	hiala Configuration	Not Provide	4							
VCI	Cargo Pody Typo	Not Provide	4					Liconso Class		
	CVWP/CCWP	Not Provide	4				Ca	mmoroial Endorsomor	÷	
Hazardous Materia		1001100100	u				CO		ll	
	ai us Motorial Diacord							UM Class		
TIazaiuo	US Material Flacatu							HM Cargo Present		
	HM Discord Norma							HM Cargo Palaasad		
Carrier Identificati	ion							Thvi Cargo Keleased		
	Iotor Carrier Nomo									
Commencial IV	US DOT# / State									
Commonial	/Non Commercial	Not Provide	4							
Commercial	/ ivon-Commercial		u							

#### Passenger Information

EMS Transport Date of Death Position In / On Vehicle Safety Equip Used Airbag Deployment Type Ejected from Vehicle Type Injury Type



Location of First Harmful Event	3. Median	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	5. Darkness - Road Not Lighted	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	9. Fixed Object - Off Road

#### **Crash Description**

VEHICLE #1 STRUCK GUARD RAIL, LOST CONTROL THEN STRUCK GUARD RAIL AGAIN

Appendix B Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

#### 01/08/2020 12:42:08PM

# **Crash Report**

	Driver's Action	40. Fail to	Maintain Prope	r Contro	1				10 Dan Off Dead	l off	
Condition of Drive	r Contributing to	1. No Defe	ects					Vehicle Maneuver	10. Ran Oli Road -	Leit	
Driver \	vision Obscured	1. Not Obs	scured				5	Skidding Tire / Mark	4. No Visible Skid M	lark/Tire I	Mark
Type of Dri	iver Distractions	14. No Dri	ver Distraction					Vahiala Dady Type	2 Truck - Pick-up/P	assender	Truck
	Drinking	1. Had Not	t Been Drinking					venicle Body Type	2. Truck - Fick-up/F	assenger	THUCK
Method of Alcoho	ol Determination	Not Applic	cable					Vehicle Damage	8. Other		
	Drug Use	2. No						Vehicle Condition	1. No Defects		
	Driver's License					Spe	ec. Fun	ction Motor Vehicle	1. No Special Funct	ion	
Commercial	Driver's License	NotProvid	ed					EMV in service	Not Applicable		
Safety E	Equipment Used	3. Lap and	Shoulder Belt					Truck Cover	2. No		
	Air Bag	2. Not Dep	oloyed					Vehicle Disabled	Yes		
Eject	ted from Vehicle	1. Not Eje	cted			(	Comme	ercial Motor Vehicle	No		
	Date of Death							Towed	Yes		
	Injury Type	6. No Inju	ry (driver only)	Oversized					No	Cargo S	Spill No
EMS Transport NotProvided								Override	No	Under	ride No
Summons Issued 1 Ve			eu					Initial Impact Area	11. Left side - front	corner	
3	ummons issued	1. 105						Direction of Travel	North		
								Crash Events: 1.	28. Ran Off Road		
Shoo	d Poforo		Movimum		2000000	oro Ago I	Count	2.	5. Guard Rail		
Speed	rash Space	ad Limit	Safe Speed	ALL Pa				3.	5. Guard Rail		
	60 Spee	60 LIMIL	60	< ∘ ∩	0-17	0	0	4.	Not Provided		
	00	00	00	0	0	0	0	Most Harmful	5. Guard Rail		
Weight c	over 10.000 lbs	No			Seats 9	9 or more	No		Hazardous Materials	Placard	No
Commercial Motor	Vahiala Castian										
Joininerciai Wotor	venicle Section	Not Provid	led								
veni	Cargo Dody Type	Not Provid	led					Licongo Class			
		Not Provid	led				Car	License Class	÷		
Jagardaus Matarial	UV WK/UC WK	Not I lovic	ieu				COL		IL		
Hazardow	Material Discord							UM Class			
mazaruous								HM Cargo Drogent			
т	I IIVI 4-DIgit							HM Cargo Palassad			
□ Carrier Identificatio								mivi Cargo Keleased			
	11										

US DOT# / State Commercial / Non-Commercial Not Provided

#### Passenger Information

EMS Transport Date of Death Position In / On Vehicle Safety Equip Used Airbag Deployment Type Ejected from Vehicle Type Injury Type

Document Number Revised Report Crash Date City / Town of Location of Crash	183555288 0 Tuesday Salem I-81 No - At Inters	12/18/2018 section With or	835 • 0.30 Miles	Jurisdiction County of Crash Landmarks at Scene Railroad Crossing ID s North of EXIT 137	City of Salem	Mile Marker Number 137.30	GPS Lat. 37.294370 r Nur	GPS Long. -80.096820 mber of Vehicles 2	S
Crash Image				GUARD RAIL		Fat	alities Non-Pe Fatalities Pe	destrian 0 destrian 0	

- Injuries Non-Ped 0
  - Injuries Pedestrian

Crash	Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	1. Main-Line Roadway
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	4. Sideswipe - Same Direction

0

#### **Crash Description**

V1 ATTEMPTED A LANE CHANGE STRIKING V2 CAUSING SAME DIRECTION SIDESWIPE DAMAGE, V1 THEN LOST CONTROL STRIKING THE GUA ON THE LEFT SHOULDER.

THE ONLY DAMAGE TO V2 WAS THE DOT BUMPER ON THE TRAILER. TRAILER REGISTRATION: 2128646 STATE: ME VIN # 1DW1A5321DS362533

Appendix B Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

#### 01/08/2020 12:31:25PM

# **Crash Report**

Driver Informatio 09/10/1978	Age 40	Vehicle Information 1	
Driver's Action	42. Improper or Unsafe Lane	Change	14 Changing Lance
Condition of Driver Contributing to	1. No Defects	Vehicle Maneuver	14. Changing Lanes
Driver Vision Obscured	1. Not Obscured	Skidding Tire / Mark	1. Before Application of Brakes
Type of Driver Distractions	14. No Driver Distraction	) (shists Dady Type	1 Passangar car
Drinking	1. Had Not Been Drinking	venicie Body Type	I. Fassenger Car
Method of Alcohol Determination	Not Applicable	Vehicle Damage	8. Other
Drug Use	2. No	Vehicle Condition	1. No Defects
Driver's License		Spec. Function Motor Vehicle	1. No Special Function
Commercial Driver's License	No	EMV in service	Not Applicable
Safety Equipment Used	3. Lap and Shoulder Belt	Truck Cover	Not Applicable
Air Bag	2. Not Deployed	Vehicle Disabled	Yes
Ejected from Vehicle	1. Not Ejected	Commercial Motor Vehicle	No
Date of Death		Towed	Yes
Injury Type	6. No Injury (driver only)	Oversized	No Cargo Spill No
EMS Transport	No	Override	No Underride No
Summone lequed	1. Yes	Initial Impact Area	11. Left side - front corner
ourninons issued		Direction of Travel	South
		Crash Events: 1.	20. Motor Vehicle In Transport
Speed Before	Maximum Al	LL Passengers Age Count 2.	28. Ran Off Road
Crash Spee	d Limit Safe Speed	< 8 8-17 18-21 > 21 3.	5. Guard Rail
50 6	60	0 0 0 0 4.	Not Provided
Weight over 10,000 lbs	Yes	Seats 9 or more No	Hazardous Materials Placard No
Commercial Motor Vehicle Section	NY - N - 11 1		
Vehicle Configuration	Not Provided		
Cargo Body Type	Not Provided	License Class	
GVWR/GCWR	Not Provided	Commercial Endorsemen	t
Hazardous Material			
Hazardous Material Placard		HM Class	
HM 4-Digit		HM Cargo Present	
HM Placard Name		HM Cargo Released	
Carrier Identification			
Commercial Motor Carrier Name			
US DO1# / State	Not Provided		
Commercial / Non-Commercial	Not Plovided		
Passenger Information			
EMS Transport		Safety Equip Used	
Date of Death		Airbag Deployment Type	
Position In / On Vehicle		Ejected from Vehicle Type	

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 1. Going Straight Ahead Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Ty	Type of Driver Distractions 14. No Driver Distractio Drinking 1. Had Not Been Drinking				Vehicle Body Type					23. Truck - Single Unit Truck (3 Axles or More)			
Method	of Alcohol Determir	nation	Not App	licable	,				Vehicle Damage	8. Other			
	Drug	g Use	2. No						Vehicle Condition	1. No Defects			
	Driver's Lie	cense					S	pec. Fun	ction Motor Vehicle	1. No Special Funct	ion		
Com	mercial Driver's Lie	cense	Yes						EMV in service	Not Applicable			
	Safety Equipment	Used	3. Lap ar	nd Shoulder Belt					Truck Cover	1. Yes			
	Ai	ir Bag	2. Not D	eployed					Vehicle Disabled	No			
	Ejected from Ve	ehicle	1. Not Ej	jected				Comme	ercial Motor Vehicle	Yes			
	Date of I	Death							Towed	No			
	Injury	/ Туре	6. No Inj	ury (driver only)					Oversized	No	Cargo Spi	II No	
	EMS Trar	nsnort	No		Override				Override	No	Underrid	e No	
	Summons la	selled	2. No						Initial Impact Area	5. Right side - rear	corner		
	Guininons is	ssueu	2.110						Direction of Travel	South			
									Crash Events: 1.	20. Motor Vehicle Ir	n Transport		
	Speed Before			Maximum	ALL P	assend	ers Aa	e Count	2.	Not Provided			
	Crash	Spee	ed Limit	Safe Speed	< 8	8-17	18-21	> 21	3.	Not Provided			
	60		60	60	0	0	0	0	4.	Not Provided			
									Most Harmful	20. Wotor venicle in	Transport		
	Weight over 10,000	lbs	Yes			Seats	9 or mo	re No		Hazardous Materials	Placard 1	No	
Commercia	al Motor Vehicle Sec	ction											
	Vehicle Configu	iration	9. Tracto	or/Semi-trailer (Or	ne Traile	r)							
	Cargo Body	y Type	15. Other	r Cargo Body (No	t Listed Above) License Class								
	GVWR/C	GCWR	2. 10,001	1-26,000 lbs.				Cor	nmercial Endorsemen	t			
Hazardous	Material												
H	Iazardous Material P	lacard	No						HM Class				
	HM 4	-Digit							HM Cargo Present	No			
	HM Placard	Name							HM Cargo Released	No			
Carrier Ide	ntification												
Comme	ericial Motor Carrier	Name	ROARIN	NG SPRING									
	US DOT#	/ State											
Com	mercial / Non-Comn	nercial	1. Interst	ate Carrier									
_													
Passenger	Information							~	fate Danie II I				
	EMS Trans	sport						Sa	nety Equip Used				
	Date of D	eath						Airbag L	veployment Type				
	Position In / On Vel	nicle						Ejected fr	om Vehicle Type				
									Injury Type				




Fatalities Non-Pedestrian	0
Fatalities Pedestrian	0

- Injuries Non-Pedestrian 0
  - Injuries Pedestrian 0

### **Crash Information**

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	6. Traffic Lanes Marked	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	1. Rear End

#### **Crash Description**

VEHICLE 2 STOPPED IN ROADWAY DUE TO TRAFFIC AND WAS STRUCK BY VEHICLE 1.

#### 01/08/2020 12:42:08PM

## **Crash Report**

Driver Informatio	01/17/1950	Age 68	Vehicle	e Information	1	
	Driver's Action	12. Following Too Close		Vehicle M	laneuver	1 Going Straight Ahead
Condition of Drive	r Contributing to	1. No Defects		venicie iv	laneuvei	1. Coning Ortalgin / mode
Driver \	/ision Obscured	1. Not Obscured		Skidding Ti	re / Mark	4. No Visible Skid Mark/Tire Mark
Type of Dri	iver Distractions	14. No Driver Distraction		Vehicle Bo	ody Type	2. Truck - Pick-up/Passenger Truck
Mathad of Alcoho		Not Applicable		Vahiela	Damaga	8 Other
Method of Alcond		2 No		Vehicle	Condition	1 No Defects
	Driver's License	2.10	Spec	Function Moto	r Vehicle	1 No Special Function
Commercial	Driver's License	NotProvided	0000	EMV ir	n service	Not Applicable
Safety F	Enver & Electrice	3. Lap and Shoulder Belt		True	ck Cover	Not Applicable
	Air Bag	2. Not Deployed		Vehicle	Disabled	No
Eiect	ted from Vehicle	1. Not Ejected	Co	ommercial Moto	r Vehicle	No
	Date of Death	·			Towed	No
	Injury Type	6. No Injury (driver only)		0	versized	No Cargo Spill No
	ENO T	No			Override	No Underride No
~	ENIS Transport	INU 1. Vos		Initial Imp	act Area	6. Rear
S	ummons Issued	1. 108		Direction	of Travel	North
				Crash E	vents: 1.	20. Motor Vehicle In Transport
Speed	d Roforo	Maximum		hunt	2.	Not Provided
C	rash Spec	ad Limit Safe Speed		21	3.	Not Provided
	45	60 0	0 0 0 0	)	4.	Not Provided
				Most	Harmful	20. Motor Vehicle In Transport
Weight o	over 10,000 lbs	No	Seats 9 or more	No		Hazardous Materials Placard No
Commercial Motor	Vehicle Section					
Vehi	cle Configuration	Not Provided				
	Cargo Body Type	Not Provided		Lice	ense Class	
	GVWR/GCWR	Not Provided		Commercial En	ndorsemen	t
Hazardous Material						
Hazardous	s Material Placard			1	HM Class	
	HM 4-Digit			HM Carg	o Present	
H	IM Placard Name			HM Cargo	Released	
Carrier Identificatio	n					
Commericial Mc	otor Carrier Name					
_	US DOT# / State	NT ( D 111				
Commercial /	Non-Commercial	Not Provided				
Passenger Informati	on					
	EMS Transport			Safety Equip	Used	
	Date of Death		Air	bag Deployment	Туре	
Position	In / On Vehicle		Ejec	ted from Vehicle	Туре	
				Injury	Туре	

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 8. Stopped in Traffic Lane Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Ty	pe of Driver Distra	actions	14. No Dr	viver Distraction					Vahiele Dedu Ture	1 Passangar car			
Drinking 1. Ha			1. Had No	ot Been Drinking					venicle Body Type	1. Passenger car			
Method of	of Alcohol Determ	ination	Not Appli	cable	Vehicle Damage					8. Other			
	Dru	ug Use	2. No						Vehicle Condition	1. No Defects			
	Driver's L	icense			Spec. Function Motor Vehicle					1. No Special Function			
Com	mercial Driver's L	icense	NotProvid	led					EMV in service	Not Applicable			
	Safety Equipmen	t Used	3. Lap and	d Shoulder Belt					Truck Cover	Not Applicable			
	A	Air Bag	2. Not De	ployed					Vehicle Disabled	No			
	Ejected from \	/ehicle	1. Not Eje	ected				Comme	ercial Motor Vehicle	No			
Date of Deat									Towed	No			
	Injur	у Туре	6. No Inju	ry (driver only)					Oversized	No	Cargo Spill	No	
			No						Override	No	Underride	No	
	EMSTra	ansport	1NO 2. NI-						Initial Impact Area	12. Front			
Summons Issue			2. NO						Direction of Travel	North			
									Crash Events: 1.	20. Motor Vehicle In	Transport		
									2.	Not Provided			
	Speed Before			Maximum	ALL Pa	assenge	ers Age	Count	3.	Not Provided			
	Clash	Spee	ed Limit	Sale Speed	< 8	8-17	18-21	> 21	4.	Not Provided			
	U		00	0	0	0	0	0	Most Harmful	20. Motor Vehicle In	Transport		
Weight over 10,000 lbs No Commercial Motor Vehicle Section Vehicle Configuration No		No Not Provi Not Provi	ded ded		Seats 9	or more	e No	License Class	Hazardous Materials F	Placard N	0		
	GVWR/	GCWR	Not Provi	ded				Cor	nmercial Endorsemen	t			
Hazardous	Material												
Н	lazardous Material	Placard							HM Class				
	HM	4-Digit							HM Cargo Present				
	HM Placaro	d Name							HM Cargo Released				
Carrier Ide	ntification												
Comme	ricial Motor Carrie	r Name											
	US DOT#	/ State											
Comi	mercial / Non-Com	mercial	Not Provi	ded									
Passenger I	Information												
EMS Transport							Sa	afety Equip Used					
Date of Death			Airbag Denloyment Type										
	Position In / On Vehicle				Fiected from Vehicle Type								
	1 COMON 111 / OIL W						L	100100 11	Iniury Type				
									mini à rabé				

Document Number Revised Report Crash Date	192565021 0 Wednesday	09/11/2019	1730	Jurisdiction County of Crash Landmarks at Scene	Roanoke County Roanoke		GPS Lat. 37.322150	GPS Long. -80.035550	
City / Town of Location of Crash	I-81NB No - At Inters	section With o	r 0.20 Miles	Railroad Crossing ID		Mile Marker Number 141.40	r Nun	nber of Vehicle 2	es
Crash Image						Fat	alities Non-Pe Fatalities Pe ijuries Non-Pe Injuries Pe	destrian 0 destrian 0 destrian 0 destrian 0	

### Crash Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	4. Sideswipe - Same Direction

I-81NB

#### **Crash Description**

VEH 2 SLOWING FOR BACKED UP TRAFFIC (VEH'S X). VEH 1 CHANGING LANES, LOST CONTROL AND STRUCK SIDE OF VEH 2.

#### 01/08/2020 12:42:08PM

## **Crash Report**

Driver Informatio 07/15/1998	Age 21	Vehicle Information 1	
Driver's Action	40. Fail to Maintain Proper Cont	rol	14 Changing Langs
Condition of Driver Contributing to	1. No Defects	venicie Maneuver	
Driver Vision Obscured	1. Not Obscured	Skidding Tire / Mark	4. No Visible Skid Mark/Tire Mark
Type of Driver Distractions	14. No Driver Distraction	Vehicle Body Type	1. Passenger car
Method of Alcohol Determination	Not Applicable		8 Other
Drug Lleo	2 No	Vehicle Condition	1 No Defects
Driver's License	2.110	Space Euroption Mater Vehicle	1 No Special Function
Commercial Driver's License	NotProvided	EMV in service	Not Applicable
Safety Equipment Lised	3 I an and Shoulder Belt		Not Applicable
	2 Not Deployed	Vohielo Disabled	No
All Dag	1 Not Fiected	Commorcial Mater Vahiala	No
	1. 100 1. 100		No
	6 No Injury (driver only)		
injury lype	o. no injury (univer only)	Oversized	No Linderside No
EMS Transport	No		9 Left side - middle
Summons Issued	1. Yes	Initial Impact Area	North
			20 Motor Vohiolo In Transport
		Crash Events: 1.	Not Provided
Speed Before	Maximum ALL	Passengers Age Count	Not Provided
Crash Spee	ed Limit     Safe Speed      < 8	3 8-17 18-21 > 21	Not Provided
50	60 40 2	0 0 1 4.	20 Meter Vehicle In Transport
Weight over 10,000 lbs Commercial Motor Vehicle Section Vehicle Configuration Cargo Body Type GVWR/GCWR Hazardous Material	No Not Provided Not Provided	Seats 9 or more <b>No</b> License Class Commercial Endorsement	Hazardous Materials Placard <b>No</b>
Hazardous Material Placard		HM Class	
HM 4-Digit		HM Cargo Present	
HM Placard Name		HM Cargo Released	
Carrier Identification			
Commericial Motor Carrier Name			
US DOT# / State			
Commercial / Non-Commercial	Not Provided		
Passenger Information			
EMS Transport		Safety Equip Used	
Date of Death		Airbag Deployment Type	
Position In / On Vehicle		Ejected from Vehicle Type Injury Type	
Driver Information 07/17/1991	Age 28	Vehicle Information 2	

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 5. Slowing or Stopping Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Type of Driver Distractions 14. No Driver Distraction Drinking 1. Had Not Been Drinking					Vehicle Body Type 2. Truck - Pick-up/Passenger Truck						Truck	
Method of	of Alcohol Determin	nation	Not Appli	icable					Vehicle Damage	8. Other		
	Dru	g Use	2. No				-	_	Vehicle Condition	1. No Defects		
0	Driver's Li	cense	N-4D	L _ L			S	bec. Fun	iction Motor Vehicle	Not Applicable		
Com	mercial Driver's Li	cense	NotProvid	ded		EMV in service						
	Safety Equipment	Used	3. Lap and	a Shoulder Bell					Iruck Cover	Z. NO		
	A	Ir Bag	2. Not De	pioyed	Venicie Disabled				venicle Disabled	No		
	Elected from V	enicie	1. NOLEJ	ected	Commercial Motor Venicle			No				
	Date of	Death	6 No Ini	uru (drivar anlu)					Iowed	No	0.00000	
	injury	/ Type	0. NO IIIJU	ing (univer only)					Oversized	No	Cargo S	pili No
	EMS Trai	nsport	No						Override	1 Dight side rear	Underr	ide NO
	Summons I	ssued	2. No						Discotion of Travel	4. Night Side - Teal		
									Direction of Travel	20 Motor Vohicle In	Transpor	+
									Crash Events: 1.	Not Provided	папъро	L
	Speed Before			Maximum	ALL Pa	assenge	ers Age	e Count	2.	Not Provided		
	Crash	Spee	ed Limit	Safe Speed	< 8	8-17	18-21	> 21	3.	Not Provided		
	40		60	40	0	0	0	0	4. Most Harmful	20 Motor Vehicle In	Transpor	+
Commercia	Weight over 10,000 al Motor Vehicle Sec Vehicle Configu Cargo Body	lbs etion uration v Type	No Not Provi Not Provi	ded		Seats 9	) or mo	re No	License Class	Hazardous Materials F	Placard	No
	GVWR/C	GCWR	Not Provi	ded	Commercial Endorsement							
Hazardous	Material											
Н	Iazardous Material P	Placard							HM Class			
	HM 4	4-Digit							HM Cargo Present			
	HM Placard	Name							HM Cargo Released			
Carrier Ide	ntification											
Comme	ericial Motor Carrier	Name										
	US DOT#	/ State										
Com	mercial / Non-Comn	nercial	Not Provi	ded								
Passenger 1	Information											
	EMS Transport							S	afety Equip Used			
Date of Death								Airbag I	Deployment Type			
	Position In / On Vel	hicle					1	Ejected fi	rom Vehicle Type Injury Type			



### Crash Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	1. Main-Line Roadway
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	3. Grade - Straight	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	4. Sideswipe - Same Direction

#### **Crash Description**

VEHICLE 2 WAS TRAVELING NORTH IN THE LEFT LANE. VEHICLE 1 CHANGED LANES FROM RIGHT TO LEFT AND STRUCK VEHICLE 2.

#### 01/08/2020 12:26:56PM

## Crash Report

Driver Informatio 02/12/1962	Age 57	Vehicle Information 1				
Driver's Action	42. Improper or Unsafe Lane	Change	14 Changing Lanos			
Condition of Driver Contributing to	1. No Defects	venicie Maneuver	14. Changing Lanes			
Driver Vision Obscured	1. Not Obscured	Skidding Tire / Mark	4. No Visible Skid Mark/Tire Mark			
Type of Driver Distractions	14. No Driver Distraction	Vahiala Rady Typa	4 Truck - Single Unit Truck (2-Axles)			
Drinking	1. Had Not Been Drinking	venicle body Type	4. Truck - Olligie Olik Truck (2-Axies)			
Method of Alcohol Determination	Not Applicable	Vehicle Damage	8. Other			
Drug Use	2. No	Vehicle Condition	1. No Defects			
Driver's License		Spec. Function Motor Vehicle	1. No Special Function			
Commercial Driver's License	No	EMV in service	Not Applicable			
Safety Equipment Used	3. Lap and Shoulder Belt	Truck Cover	Not Applicable			
Air Bag	2. Not Deployed	Vehicle Disabled	No			
Ejected from Vehicle	1. Not Ejected	Commercial Motor Vehicle	Yes			
Date of Death		Towed	No			
Injury Type	6. No Injury (driver only)	Oversized	No Cargo Spill No			
FMS Transport	No	Override	No Underride No			
Summons Issued	1. Yes	Initial Impact Area	8. Left side - rear			
		Direction of Travel	North			
		Crash Events: 1.	20. Motor Vehicle In Transport			
Speed Before	Maximum A	2. 2.	Not Provided			
Crash Spec	ed Limit Safe Speed	< 8 8-17 18-21 > 21	Not Provided			
65	60 60	0 0 0 0 4.	Not Provided			
		Most Harmful	20. Motor Vehicle In Transport			
Weight over 10,000 lbs	Yes	Seats 9 or more No	Hazardous Materials Placard No			
Commercial Motor Vehicle Section						
Vehicle Configuration	5. Single Unit Truck (2 Axle	s, 6 Tires)				
Cargo Body Type	3. Van/Enclosed Box	License Class				
GVWR/GCWR	2. 10,001-26,000 lbs.	Commercial Endorsemen	t			
Hazardous Material						
Hazardous Material Placard	No	HM Class				
HM 4-Digit		HM Cargo Present	No			
HM Placard Name		HM Cargo Released	No			
Carrier Identification						
Commericial Motor Carrier Name	MOUNT AIRY					
US DOT# / State						
Commercial / Non-Commercial	1. Interstate Carrier					
Passenger Information						
EMS Transport		Safety Equip Used				
Date of Death		Airbag Deployment Type				
Position In / On Vehicle		Ejected from Vehicle Type				
		Injury Type				

Driver Informatio 09/11/1988 Age 31

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 1. Going Straight Ahead Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Vehicle Information 2

Ty	pe of Driver Distra	actions	14. No Di	river Distraction					Vehicle Body Type	1. Passenger car		
	D	rinking	1. Had No	ot Been Drinking					Veniele Body Type			
Method o	of Alcohol Determi	ination	Not Appli	cable					Vehicle Damage	8. Other		
	Dru	ig Use	2. No						Vehicle Condition	1. No Defects		
	Driver's L	icense				Spec. Function Motor Vehicle				1. No Special Function		
Com	mercial Driver's L	icense	NotProvid	led	EMV in service					Not Applicable		
	Safety Equipmen	t Used	3. Lap and	d Shoulder Belt	Truck Cover					Not Applicable		
	A	Air Bag	2. Not De	ployed	Vehicle Disabled					Yes		
	Ejected from \	/ehicle	1. Not Eje	ected				Comm	ercial Motor Vehicle	No		
	Date of	Death							Towed	Yes		
	Injur	у Туре	6. No Inju	ary (driver only)					Oversized	No	Cargo Spill	No
	EMS Tra	nsport	No						Override	No	Underride	No
	Summons	lesued	2. No						Initial Impact Area	3. Right side - midd	le	
	ourninons	155000	2.110						Direction of Travel	North		
									Crash Events: 1.	20. Motor Vehicle Ir	า Transport	
	Speed Before			Maximum		assona	ors Ado	Count	2.	Not Provided		
	Crash	Snor	od Limit	Safe Speed	~ Q	23361190 0 17	10 01	> 21	3.	Not Provided		
	70	oped	60	60	0	0	0	0	4.	Not Provided		
	10		00	00	Ū	U	0	0	Most Harmful	20. Motor Vehicle Ir	า Transport	
Commercia	Weight over 10,000 al Motor Vehicle Se	) lbs	Yes	dad		Seats 9	9 or mor	e No		Hazardous Materials	Placard N	lo
	Venicle Config	uration	Not Provi	dad								
	Cargo Boo		Not Provi	dad				C				
TT	GVWK/	GUWK	100111001	ucu				Co	mmercial Endorsemen	l		
Hazardous	Material	Dlagard							UN Class			
П	azardous Material								HIVI Class			
		4-Digit							HM Cargo Present			
Comi en Lio		i Name							HM Cargo Keleased			
	numeation	- NI										
Comme												
C	US DOT#	·/ State	Not Provi	dad								
Com	mercial / Non-Com	mercial	NOT FIOVE	ueu								
Dassonaarl	nformation											
I assenger I	EMS Tran	sport						ç	a faty Equip Used			
	Data of I	Death						O Airbaa I	Deployment Type			
	Date 01 1	hicle					E	incted f	rom Vehicle Type			
		mere					E	lected I	Inium Type			
									IIIIII Y IYPC			





Injuries Non-Pedestrian 0

Injuries Pedestrian 0

### Crash Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	3. Grade - Straight	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	2. Two-Way, Divided, Unprotected Median	Type of Collision	1. Rear End

### **Crash Description**

VEHICLE #2 SLOWED FOR TRAFFIC AHEAD, VEHICLE #1 STRUCK THE TRAILER THAT VEHICLE #2 WAS TOWING. DRIVER OF VEHICLE #1 ADVISED THAT HE WAS LOOKING AT HIS MIRROR ON THE RIGHT WHEN HE STRUCK THE OTHER VEHICLE.

#### 01/08/2020 12:31:25PM

## **Crash Report**

Driver information 01/	/09/1984	Age 34	Vehic	le Information 1	
Di	river's Action	12. Following Too Close		Vehicle Maneuve	r 1. Going Straight Abead
Condition of Driver Co	ontributing to	1. No Defects		Obidalia a Tina / Mard	A No Visible Skid Mark/Tire Mark
Type of Driver	Distractions	11. Other		Skidding Tire / Mari	
.,	Drinking	1. Had Not Been Drinking		Vehicle Body Type	23. Truck - Single Unit Truck (3 Axles or More
Method of Alcohol D	etermination	Not Applicable		Vehicle Damage	8. Other
	Drug Use	2. No	_	Vehicle Condition	1. No Defects
Driv	ver's License	V	Spe	c. Function Motor Vehicle	1. No Special Function
Commercial Driv	/er's License	Yes 2 Lan and Shouldar Palt		EMV in service	
Satety Equi	Ipment Used	2. Not Deployed		Iruck Cove	
Figstad	AIr Bag	<ol> <li>Not Deployed</li> <li>Not Ejected</li> </ol>		Venicle Disabled	Ves
Elected		1. Not Ejected	C		No
D		6 No Injury (driver only)		Oversized	NO Cargo Spill No
	Injury Type	o. No injury (uriver only)		Oversized	No Cargo Spill No
EN	/IS Transport	No		Initial Impact Area	12 Front
Sumr	mons Issued	1. Yes		Direction of Trave	South
				Crash Events: 1	20 Motor Vehicle In Transport
				2	Not Provided
Speed Be	efore	Maximum	ALL Passengers Age (	count 3	Not Provided
Crash	h Spee	ed Limit Safe Speed	< 8 8-17 18-21 3	21	Not Provided
65		60 60	0 0 0	0 Most Harmfu	20. Motor Vehicle In Transport
Weight over Commercial Motor Veh Vehicle	10,000 lbs icle Section Configuration	No Not Provided	Seats 9 or more	No	Hazardous Materials Placard No
Car	go Body Type	Not Provided		License Clas	S
G	VWR/GCWR	Not Provided		Commercial Endorseme	nt
Hazardous Material				micol	
Hazardous Ma	aterial Placard			HM Class	S
	HM 4-Digit			HM Cargo Presen	1
HIM I	Placard Name			HIM Cargo Released	1
Commonication	Carrier Name				
	DOT# / State				
Commercial / Nor	n-Commercial	Not Provided			
Passenger Information					
EM	IS Transport			Safety Equip Used	
Da	ate of Death		А	irbag Deployment Type	
Position In /	On Vehicle		Eje	cted from Vehicle Type Injury Type	
	/20/1000	20		to be formulation =	

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 5. Slowing or Stopping Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Ty	pe of Driver Distra	ctions	14. No Di	river Distraction					Valaiala Dadu Tura	1 Truck Single I	l Init Truck (	2 1	oc)
	Dr	inking	1. Had No	ot Been Drinking					venicie body Type	4. Huck - Olingie		2-77	63)
Method of	of Alcohol Determi	nation	Not Appl	icable					Vehicle Damage	8. Other			
	Dru	g Use	2. No						Vehicle Condition	1. No Defects			
	Driver's Li	cense					S	pec. Fur	ction Motor Vehicle	1. No Special Fur	nction		
Com	mercial Driver's Li	cense	Yes						EMV in service	Not Applicable			
	Safety Equipment	Used	3. Lap an	d Shoulder Belt					Truck Cover	Not Applicable			
	A	ir Bag	2. Not De	ployed					Vehicle Disabled	No			
	Ejected from V	ehicle	1. Not Eje	ected				Comm	ercial Motor Vehicle	No			
	Date of	Death							Towed	No			
	Injury	/ Туре	6. No Inju	ury (driver only)					Oversized	No	Cargo S	Spill	No
			Na						Override	No	Under	ride	No
	EIVIS Trai	nsport	NO 2 No						Initial Impact Area	6. Rear			
	Summons	ssued	2. INO						Direction of Travel	South			
									Crash Events: 1.	20. Motor Vehicle	In Transpo	ort	
	On and Defense			N.4				0	2.	Not Provided			
	Speed Before	0		Maximum	ALL Pa	assenge	ers Age		3.	Not Provided			
	60	Spee	ed Limit		< 8	8-17	18-21	> 21	4.	Not Provided			
	00		00	00	0	0	0	0	Most Harmful	20. Motor Vehicle	In Transpo	ort	
Commercia	Weight over 10,000 al Motor Vehicle Sec Vehicle Config	lbs ction	No Not Provi	ded		Seats 9	or mo	re No		Hazardous Materia	ls Placard	No	
	Cargo Body	v Type	Not Provi	ided					License Class				
	GVWP/	CWP	Not Provi	ided				Co	nmarcial Endorsaman	ŀ			
Hazardous	Material		110111011	lucu				CO	inneretai Endorsemen	L			
H	lazardous Material F	lacard							HM Class				
11	HM /	I_Digit							HM Cargo Present				
	HM Placard	Name							HM Cargo Released				
Carrier Ide	ntification	Ivanie							Thir Cargo Released				
Comme	ricial Motor Carrier	Name											
Comme	US DOT#	/ State											
Com	mercial / Non-Comr	nercial	Not Provi	ided									
Com		liereiur											
Dassanger	Information												
I assenger i	EMS Tran	nort						S.	afety Equip Used				
		)ooth						Airbag I	Perloyment Type				
	Position In / On Val	hicle					1	Fiected fo	om Vehicle Type				
	i ositioli ili / Oli ve	mere					1	Elected II	Injury Type				
									mun inde				

0

# Crash Report





### Crash Information

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	2. Two-Way, Divided, Unprotected Median	Type of Collision	1. Rear End

### **Crash Description**

VEHICLE #2 STOPPED FOR TRAFFIC, VEHICLE #1 STRUCK VEHICLE #2 IN THE REAR.

## **Crash Report**

Driver Informatio	07/03/1996	Age 22			Vehi	cle Info	rmation	1			
Condition of Drive	Driver's Action er Contributing to	12. Following Too Close 1. No Defects					Vehicle M	laneuver	1. Going Straight Ah	ead	
Driver	Vision Obscured	1. Not Obscured				S	skidding Ti	re / Mark	4. No Visible Skid Ma	ark/Tire Mar	ĸ
Type of D	river Distractions Drinking	14. No Driver Distraction 1. Had Not Been Drinking					Vehicle B	ody Type	1. Passenger car		
Method of Alcoh	nol Determination	Not Applicable					Vehicle	Damage	6. Totaled		
	Drug Use	2. No					Vehicle (	Condition	1. No Defects		
	Driver's License				Spe	ec. Fun	ction Moto	or Vehicle	1. No Special Function	on	
Commercia	I Driver's License	NotProvided					EMV i	n service	Not Applicable		
Safety	Equipment Used	3. Lap and Shoulder Belt					Tru	ck Cover	Not Applicable		
	Air Bag	1. Deployed - Front					Vehicle	Disabled	Yes		
Eje	cted from Vehicle	1. Not Ejected				Comme	ercial Moto	r Vehicle	No		
	Date of Death							Towed	Yes		
	Injury Type	6. No Injury (driver only)					С	Versized	No	Cargo Spill	No
	EMC Transport	No						Override	No	Underride	No
	EIVIS Transport	INU 1. Vec					Initial Imp	oact Area	12. Front		
	Summons Issued	1. 105					Direction	of Travel	North		
							Crash E	vents: 1.	20. Motor Vehicle In	Transport	
Creati	ad Defens	Maximum				Count	]	2.	Not Provided		
Spee	Prash Crosh	Maximum Safe Speed	ALL Pass	senge	rs Age			3.	Not Provided		
	siasii Spee		< 8 0	5-17	18-21	> 21		4.	Not Provided		
	00	0	0	0	0	0	Most	t Harmful	20. Motor Vehicle In	Transport	
Weight	over 10,000 lbs	No	S	leats 9	or more	No			Hazardous Materials P	lacard N	0
Commercial Motor	r Vehicle Section										
Vel	nicle Configuration	Not Provided									
	Cargo Body Type	Not Provided					Lice	ense Class			
	GVWR/GCWR	Not Provided				Con	nmercial Er	ndorsement	t		
Hazardous Materia	ıl										
Hazardou	us Material Placard							HM Class			
	HM 4-Digit						HM Carg	go Present			
	HM Placard Name						HM Cargo	Released			
Carrier Identificati	on										

Commercial Motor Carrier Name US DOT# / State Commercial / Non-Commercial Not Provided

### Passenger Information

EMS Transport Date of Death Position In / On Vehicle Safety Equip Used Airbag Deployment Type Ejected from Vehicle Type Injury Type

2

Vehicle Information

Driver Informatio 12/23/1982

Age 35

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 8. Stopped in Traffic Lane Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Ty	pe of Driver Distra	ctions	14. No Di	river Distraction					Vahiele Dedu Ture	22 Truck Sport Lit	ility Vohielo	(911)/)
	Dri	inking	1. Had No	ot Been Drinking					venicie body Type	22. Huck - Opon Ot	inty vernicie	(507)
Method of	of Alcohol Determin	nation	Not Appl	icable					Vehicle Damage	8. Other		
	Dru	g Use	2. No						Vehicle Condition	1. No Defects		
	Driver's Lie	cense					S	pec. Fur	action Motor Vehicle	1. No Special Funct	ion	
Com	mercial Driver's Lie	cense	NotProvi	ded					EMV in service	Not Applicable		
	Safety Equipment	Used	3. Lap an	d Shoulder Belt					Truck Cover	Not Applicable		
	A	ir Bag	2. Not De	eployed					Vehicle Disabled	No		
	Ejected from V	ehicle	1. Not Eje	ected				Comm	ercial Motor Vehicle	No		
	Date of I	Death							Towed	No		
	Injury	туре	6. No Inji	ury (driver only)					Oversized	No	Cargo Spi	II No
	EMO Tro		No						Override	No	Underrid	e No
	EIVIS Trar	ISPOR	1N0 2 No						Initial Impact Area	6. Rear		
	Summons	ssued	2. 10						Direction of Travel	North		
									Crash Events: 1.	20. Motor Vehicle In	n Transport	
	Created Defense			Marriene				Carriet	2.	Not Provided		
	Speed Belore	0	a al I ina it	Iviaximum Safe Speed	ALL Pa	assenge	ers Age		3.	Not Provided		
	0	Spee	ea Limit 60		< 8 0	0-17	18-21	> 21	4.	Not Provided		
	0		00	0	0	0	0	0	Most Harmful	20. Motor Vehicle In	n Transport	
Commercia	Weight over 10,000 al Motor Vehicle Sec	lbs	No			Seats 9	or mo	re No		Hazardous Materials	Placard 1	No
	Vehicle Configu	iration	Not Provi									
	Cargo Body	y Type	Not Provi	ided				~	License Class			
** 1	GVWR/C	<i>i</i> CWR	Not Provi	ided				Coi	mmercial Endorsemen	t		
Hazardous	Material											
H	lazardous Material P	lacard							HM Class			
	HM 4	-Digit							HM Cargo Present			
~	HM Placard	Name							HM Cargo Released			
Carrier Ide	ntification	<b>.</b> .										
Comme	ericial Motor Carrier	Name										
~	US DOT#	/ State	NUD									
Com	mercial / Non-Comn	nercial	Not Provi	ided								
<b>D</b>												
Passenger	Information							a	fata E min Unad			
	ENIS Trans	sport						Si Si	allely Equip Used			
	Date of D	eath					,	Airbag I	Deployment Type			
	Position In / On Vel	nicle						Ejected fi	om Vehicle Type			
									T - 70			
									Injury Type			

Document Number172835338Revised Report1Crash DateWednesdayCity / Town ofINTERSTATLocation of CrashINTERSTATNo - At Inter	10/04/2017 1315 E 81 section With or 0.10 Miles	Jurisdiction County of Crash Landmarks at Scene Railroad Crossing ID North of ROUTE 112	Roanoke County Roanoke Mile I	Marker Number 137.20	GPS Lat. G 37.295190 - Numbe	PS Long. 80.092950 er of Vehicles 1
Crash Image	DITCH	INTERSTATE 81	5	Fatali I Inju	ities Non-Pedes Fatalities Pedes Iries Non-Pedes Injuries Pedes	strian 0 strian 1 strian 0
Crash Information						
Location of First Harmful Eve	n 1 No Adverse Conditio	n (Clear/Cloudy)	Roadway Defects	8 Non-Intersec	tion	
Veather Conditio	n 2 Davlight	(Cical/Cicucy)		1 Not at Interse	ection	
Traffic Control Mechanical Dovid	a 1 Yes - Working		Work Zopo Polotod	2 No		
	e 17. Highway Safety Co	rridor V	/ork Zone Workers Present	Not Provided		

**Crash Description** 

Roadway Alignment 1. Straight - Level

VEHICLE 1 RAN OFF ROAD TO LEFT HITTING A DITCH.

Roadway Surface Type 2. Blacktop, Asphalt, Bituminous

Roadway Description 3. Two-Way, Divided, Positive Median Barrier

Roadway Surface Condition 1. Dry

Work Zone Location Not Provided

School Zone 3. No

Work Zone Type Not Provided

Type of Collision 9. Fixed Object - Off Road

#### 01/08/2020 12:26:56PM

## **Crash Report**

Driver Informatio 0	3/07/1941	Age 76				Vehi	cle Info	ormation 1				
[	Driver's Action	40. Fail to	Maintain Prope	r Contro	l			Vahiala Manauwar	10 Ran Off Road	l oft		
Condition of Driver (	Contributing to	6. Fatigue	d					venicie waneuver	IU. IVAII OII IVUAU	Len		
Driver Vis	sion Obscured	1. Not Ob	scured				S	Skidding Tire / Mark	4. No Visible Skid	Mark/Tire	Mark	<b>(</b>
Type of Drive	er Distractions	2. Driver I	Fatigue					Vehicle Body Type	1 Passenger car			
	Drinking	1. Had No	t Been Drinking					venicie body Type	n r dooongor our			
Method of Alcohol	Determination	Not Appli	cable					Vehicle Damage	8. Other			
	Drug Use	2. No						Vehicle Condition	1. No Defects			
Dr	river's License					Spe	ec. Fun	ction Motor Vehicle	1. No Special Fund	ction		
Commercial Dr	river's License	NotProvid	led					EMV in service	Not Applicable			
Safety Eq	uipment Used	3. Lap and	l Shoulder Belt					Truck Cover	Not Applicable			
	Air Bag	2. Not Dep	ployed					Vehicle Disabled	Yes			
Ejecteo	d from Vehicle	1. Not Eje	cted			(	Comme	ercial Motor Vehicle	No			
	Date of Death							Towed	Yes			
	Injury Type	3. Minor/I	Possible Injury					Oversized	No	Cargo	Spill	No
F	MS Transport	Ves						Override	No	Under	ride	No
Sur	mone lequed	1 Yes						Initial Impact Area	12. Front			
Sui	IIIIolis Issued	1. 105						Direction of Travel	North			
								Crash Events: 1.	28. Ran Off Road			
Sneed I	Refore		Maximum		esenae		Count	2.	14. Ditch			
Cra	sh Snee	ed L imit	Safe Speed	< 8	8-17	18-21	> 21	3.	Not Provided			
60	)	60	60	0	0	0	0	4.	Not Provided			
	-					_		Most Harmful	14. Ditch			
Weight over	er 10,000 lbs	No			Seats 9	or more	No		Hazardous Materials	Placard	No	3
Commercial Motor Ve	hicle Section											
Vehicle	e Configuration	Not Provid	ded									
Ca	argo Body Type	Not Provid	ded					License Class				
(	GVWR/GCWR	Not Provid	ded				Cor	nmercial Endorsemen	t			
Hazardous Material												
Hazardous N	Aaterial Placard							HM Class				
	HM 4-Digit							HM Cargo Present				
HM	1 Placard Name							HM Cargo Released				
1111												

Commericial Motor Carrier Name US DOT# / State Commercial / Non-Commercial Not Provided

### Passenger Information

EMS Transport Date of Death Position In / On Vehicle Safety Equip Used Airbag Deployment Type Ejected from Vehicle Type Injury Type





ata	lities	Non-	-Pe	destri	an	U
	Fata	lities	Pe	destri	an	ſ

- Fatalities Pedestrian 0
- Injuries Non-Pedestrian 0
  - Injuries Pedestrian 0

### **Crash Information**

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	17. Highway Safety Corridor	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	1. Rear End

#### **Crash Description**

VEHICLE 2 STOPPED IN ROADWAY DUE TO TRAFFIC AND WAS STRUCK BY VEHICLE 1. VEHICLE 2 WAS PUSHED INTO THE SIDE OF VEHICLE 3 SPUN AROUND COMING TO A REST IN THE RIGHT LANE FACING BACK SOUTH.

#### 01/08/2020 12:26:56PM

## Crash Report

Driver Informatio 1	2/29/1987	Age 30	Veh	icle Information	1		
I	Driver's Action	12. Following Too Close		Vehicle	Maneuver	1. Going Straight Ahead	
Condition of Driver (	Contributing to	1. No Defects		01.1.1.		2 After Application of Brokes	
Driver Vis	sion Obscured	<ol> <li>Not Obscured</li> <li>Eves Not on Road</li> </ol>		Skidding	Tire / Mark	2. After Application of Brakes	
Type of Drive	Distractions	1. Had Not Been Drinking		Vehicle	Body Type	22. Truck - Sport Utility Vehicle	(SUV
Method of Alcohol	Determination	Not Applicable		Vehicl	e Damage	8. Other	
	Drug Use	2. No		Vehicle	Condition	1. No Defects	
Dr	river's License		Sp	ec. Function Mo	tor Vehicle	1. No Special Function	
Commercial Dr	river's License	NotProvided		EMV	/ in service	Not Applicable	
Safety Eq	uipment Used	3. Lap and Shoulder Belt		Tr	ruck Cover	Not Applicable	
	Air Bag	1. Deployed - Front		Vehicle	e Disabled	Yes	
Ejected	d from Vehicle	1. Not Ejected		Commercial Mo	tor Vehicle	No	
	Date of Death	<b></b>			Towed	Yes	
	Injury Type	6. No Injury (driver only)			Oversized	NO Cargo Spi	No
E	MS Transport	No			Override	No Underride	e No
Sur	nmons Issued	1. Yes		Initial In	npact Area	12. Front	
				Directio	n of Travel	20 Motor Vohicle In Transport	
				Crash	Events: 1.	Not Provided	
Speed I	Before	Maximum	ALL Passengers Age	Count	2.	Not Provided	
Cra	sh Spee	ed Limit Safe Speed	< 8 8-17 18-21	> 21	З. Д	Not Provided	
55	5	60 0	0 0 0	0	+. set Hormful	20 Motor Vehicle In Transport	
Commercial Motor Ve	chicle Section	165	Seats 9 or more	- INO		Hazardous Materiais Placard	NU
Vehicle	e Configuration	Not Provided					
Ca	argo Body Type	Not Provided		Li	cense Class		
(	GVWR/GCWR	Not Provided		Commercial	Endorsemen	nt	
Hazardous Material							
Hazardous N	Aaterial Placard			mic	HM Class		
	HM 4-Digit			HM Ca	argo Present		
HIV. Corrier Identification	1 Placard Name			HM Car	go Keleased		
Commercial Moto	or Carrier Name						
	S DOT# / State						
Commercial / No	on-Commercial	Not Provided					
Passenger Information	1						
E	MS Transport			Safety Equi	ip Used		
1	Date of Death			Airbag Deploymer	nt Type		
Position In	n / On Vehicle		Ε	jected from Vehic	le Type		
				Inju	ry Type		
Driver Informatio 0	8/07/1987	Age 31	Veh	icle Information	2		

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 8. Stopped in Traffic Lane Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Type of Driver Distractions	14. No Driver Distraction	Vehiala Pady Type	1 Passenger car
Drinking	1. Had Not Been Drinking	Venicle Dody Type	
Method of Alcohol Determination	Not Applicable	Vehicle Damage	6. Totaled
Drug Use	2. No	Vehicle Condition	1. No Defects
Driver's License		Spec. Function Motor Vehicle	1. No Special Function
Commercial Driver's License	NotProvided	EMV in service	Not Applicable
Safety Equipment Used	3. Lap and Shoulder Belt	Truck Cover	Not Applicable
Air Bag	8. Deployed - Combination	Vehicle Disabled	Yes
Fiected from Vehicle	1. Not Ejected	Commercial Motor Vehicle	No
		Towed	Yes
	6 No Injury (driver only)	Oversized	No Cargo Spill No
	o. No injury (uriver only)	Oversized	No Linderride No
EMS Transport	No		6 Rear
Summons Issued	2. No	Initial Impact Area	North
		Direction of Travel	Norui
		Crash Events: 1.	20. Motor Venicle in Transport
Speed Before	Maximum	ALL Passengers Age Count 2.	20. Motor Vehicle In Transport
Crash Spe	ed Limit Safe Speed	< 8 8-17 18-21 > 21 3.	Not Provided
0	60 0	0 0 0 0 4.	Not Provided
-		Most Harmful	20. Motor Vehicle In Transport
Weight over 10 000 lbs	Yes	Seats 9 or more No	Hazardous Materials Placard No
Weight 6Vei 10,000 103		Source and the second	
Commercial Motor Vehicle Section			
Vehicle Configuration	Not Provided		
Cargo Body Type	Not Provided	License Class	
GVWR/GCWR	Not Provided	Commercial Endorsemen	t
Hazardous Material			
Hazardous Material Placard		HM Class	
HM 4-Digit		HM Cargo Present	
HM Placard Name		HM Cargo Released	
Carrier Identification			
Commericial Motor Carrier Name			
US DOT# / State			
Commercial / Non-Commercial	Not Provided		
Commercial / Non-Commercial	10011001404		
Passenger Information			
EMS Transport		Safety Equip Used	
Date of Death		Airbag Deployment Type	
Position In / On Vehicle		Ejected from Vehicle Type	
		Injury Type	
		interv rype	
Driver Information 03/10/1957	Ago 61	Vahiala Information 3	
	Aye or		
Driver's Action	1. No Improper Action		
Condition of Driver Contributing to	1. No Defects	Vehicle Maneuver	1. Going Straight Anead
Driver Vision Obscured	1. Not Obscured	Skidding Tire / Mark	4. No Visible Skid Mark/Tire Mark
Type of Driver Distractions	14. No Driver Distraction		·
Drinking	1. Had Not Been Drinking	Vehicle Body Type	23. Truck - Single Unit Truck (3 Axles or More)
Method of Alcohol Determination	Not Applicable	Vahiala Damaga	8 Other
	2 No	Vehicle Damage	1 No Defects
Drug Use	2. INU	Venicle Condition	1. No Special Eurotion
Driver's License	X	Spec. Function Motor Vehicle	
Commercial Driver's License	res	EMV in service	
Safety Equipment Used	3. Lap and Shoulder Belt	Truck Cover	NOT Applicable
Air Bag	<ol><li>Not Deployed</li></ol>	Vehicle Disabled	No

Crash300

Appendix B

Commonwealth of Virginia -- Department of Transportation -- Traffic Engineering Division

		I		
Ejected from Vehic	le 1. Not Ejected	Comr	nercial Motor Vehicle	Yes
Date of Dea	th		Towed	No
Injury Typ	6. No Injury (driver only)		Oversized	No Cargo Spill No
EMS Transpo	vrt No		Override	No Underride No
EIMS TRAISPO	ad 2 No		Initial Impact Area	8. Left side - rear
Summons issue	2.10		Direction of Travel	North
			Crash Events: 1.	20. Motor Vehicle In Transport
Speed Before	Maximum	ALL Passengers Age Cours	2.	Not Provided
Crash Sr	Safe Speed	< 8 8-17 18-21 > 21	3.	Not Provided
25	60 0	0 0 0 0	4.	Not Provided
			Most Harmful	20. Motor Vehicle In Transport
Weight over 10,000 lbs Commercial Motor Vehicle Section Vehicle Configuration	Yes 9. Tractor/Semi-trailer (Or	Seats 9 or more No		Hazardous Materials Placard No
Cargo Body Ty	e 8. Auto Transporter		License Class	
GVWR/GCW	$\mathbb{R}$ 3. Greater Than 26,000 lbs	s. C	ommercial Endorsement	t
Hazardous Material				
Hazardous Material Placa	rd No		HM Class	N-
HM 4-Dis	zit.		HM Cargo Present	No
HM Placard Nan	ne		HM Cargo Released	NO
Carrier Identification	TUPELO			
	ta			
Commercial / Non Commercial	al 1 Interstate Carrier			
Commercial / Ivon-Commercial				
Passanger Information				
FMS Transport			Safety Equip Used	
Date of Death		Airhao	Deployment Type	
Position In / On Vehicle		Eiected	from Vehicle Type	
		Elected	Injury Type	





Fatalities Non-Pedestrian	0
Fatalities Pedestrian	0
Injuries Non-Pedestrian	0
Injuries Pedestrian	0

Location of First Harmful Event	1. On Roadway	Roadway Defects	1. No Defects
Weather Condition	1. No Adverse Condition (Clear/Cloudy)	Relation to Roadway	8. Non-Intersection
Light Condition	2. Daylight	Intersection Type	1. Not at Intersection
Traffic Control Mechanical Device	1. Yes - Working	Work Zone Related	2. No
Traffic Control Type	6. Traffic Lanes Marked	Work Zone Workers Present	Not Provided
Roadway Alignment	1. Straight - Level	Work Zone Location	Not Provided
Roadway Surface Condition	1. Dry	Work Zone Type	Not Provided
Roadway Surface Type	2. Blacktop, Asphalt, Bituminous	School Zone	3. No
Roadway Description	3. Two-Way, Divided, Positive Median Barrier	Type of Collision	2. Angle

#### **Crash Description**

VEHICLE # 2 HAD STOPPED FOR TRAFFIC, VEHICLE # 1 ATTEMPTED TO STOP LOST CONTROL AND STRUCK VEHICLE # 2.

#### 01/08/2020 12:26:56PM

## **Crash Report**

Driver Informatio 06/29/	/1984	Age 34				Veh	nicle Info	ormation	1		
Drive	er's Action	40. Fail to	Maintain Prope	er Control	l					1 Coing Straight Aboad	
Condition of Driver Contr	ributing to	1. No Defe	ects					Vehicle	Maneuver	L. Going Straight Anead	
Driver Vision	Obscured	1. Not Obs	scured					Skidding ⁻	Tire / Mark	4. No Visible Skid Mark/Tire Ma	ark
Type of Driver Di	stractions	14. No Dri	iver Distraction					Vohielo	Body Typo	1 Passenger car	
	Drinking	1. Had No	t Been Drinking	ş				Venicie	Douy Type	1. Tubboligor bui	
Method of Alcohol Dete	rmination	Not Applie	cable					Vehicl	e Damage	8. Other	
	Drug Use	2. No						Vehicle	Condition	1. No Defects	
Driver	s License					Sp	ec. Fur	nction Mo	tor Vehicle	1. No Special Function	
Commercial Driver	s License	NotProvid	ed					EMV	' in service	Not Applicable	
Safety Equipm	nent Used	3. Lap and	l Shoulder Belt					Tr	uck Cover	Not Applicable	
	Air Bag	2. Not Dep	ployed					Vehicle	e Disabled	No	
Ejected from	m Vehicle	1. Not Eje	cted				Comm	ercial Mo	tor Vehicle	No	
Date	e of Death								Towed	No	
In	ijury Type	6. No Inju	ry (driver only)						Oversized	No Cargo Sp	ill No
EMS	Transport	No							Override	No Underrid	e No
Summo	ns Issued	1. Yes						Initial In	npact Area	12. Front	
Carrier								Directio	n of Travel	North	
								Crash	Events: 1.	20. Motor Vehicle In Transport	
Speed Befor	re		Maximum	ALL Pa	ssend	ers Ade	Count		2.	Not Provided	
Crash	Sper	ed Limit	Safe Speed	< 8	8-17	18-21	> 21		3.	Not Provided	
60	opor	60	0	0	0	0	0		4.	Not Provided	
Weight over 10,	,000 lbs	No			Seats	) or mor	e No			Hazardous Materials Placard	No
Commercial Motor Vehicle	e Section										
Vehicle Cor	nfiguration	Not Provid	ded								
Cargo I	Body Type	Not Provid	ded					Li	cense Class		
GVW	/R/GCWR	Not Provid	ded				Co	mmercial 1	Endorsemen	ıt	
Hazardous Material											
Hazardous Mater	ial Placard								HM Class		
Н	IM 4-Digit							HM Ca	rgo Present		
HM Plac	card Name							HM Cars	go Released		
Carrier Identification											
Commericial Motor Car	rrier Name										
US DO	DT# / State										
Commercial / Non-Co	ommercial	Not Provid	ded								
Passenger Information											
EMS T	Transport						S	afety Equi	p Used		
Date	of Death						Airbag I	Deploymer	nt Type		
Position In / Or	n Vehicle					E	jected fi	rom Vehic	le Type		
								Injur	ry Type		
Driver Information 03/10/	/1998	Age 20				Voh	vicle Infr	ormation	2		

Driver Informatio 03/10/1998 Age 20

Driver's Action 1. No Improper Action Condition of Driver Contributing to 1. No Defects Driver Vision Obscured 1. Not Obscured

Vehicle Maneuver 8. Stopped in Traffic Lane Skidding Tire / Mark 4. No Visible Skid Mark/Tire Mark

Typ	pe of Driver Distra	actions	14. No Dr	iver Distraction					Vahiala Dady Type	1 Passenger car		
Drinking 1. Had Not Been Drinking		venicie body Type										
Method of Alcohol Determination Not Applicable		Vehicle Damage				Vehicle Damage	8. Other					
	Dru	ig Use	2. No						Vehicle Condition	1. No Defects		
	Driver's Li	icense					S	pec. Fun	action Motor Vehicle	1. No Special Funct	ion	
Com	mercial Driver's Li	icense	NotProvid	led					EMV in service	Not Applicable		
	Safety Equipment	t Used	3. Lap and	l Shoulder Belt					Truck Cover	Not Applicable		
	A	ir Bag	2. Not De	ployed					Vehicle Disabled	No		
	Ejected from V	/ehicle	1. Not Eje	cted				Comme	ercial Motor Vehicle	No		
	Date of	Death							Towed	No		
	Injur	у Туре	6. No Inju	ry (driver only)					Oversized	No	Cargo Spil	No
			No						Override	No	Underride	∋ No
	EMS Ira	nsport	NO 2 No						Initial Impact Area	4. Right side - rear		
	Summons	ssued	2. 10						Direction of Travel	North		
									Crash Events: 1.	20. Motor Vehicle In	Transport	
	On and Defense			N.4				0	2.	Not Provided		
	Speed Before	0		Maximum Safa Spood	ALL Pa	assenge	ers Age	Count	3.	Not Provided		
	0	Spee	ed Limit		< 8	0-17	18-21	> 21	4.	Not Provided		
	0		00	0	0	0	0	0	Most Harmful	20. Motor Vehicle In	Transport	
Commercia	Weight over 10,000 al Motor Vehicle Se Vehicle Config	) lbs ction uration	No Not Provi	ded		Seats 9	or moi	re No		Hazardous Materials I	Placard N	ίo
	Cargo Bod	y Type	Not Provi	ded					License Class			
	GVWR/0	GCWR	Not Provi	ded				Cor	nmercial Endorsemen	t		
Hazardous	Material											
Н	lazardous Material I	Placard							HM Class			
	HM	4-Digit							HM Cargo Present			
	HM Placard	l Name							HM Cargo Released			
Carrier Identification												
Comme	ricial Motor Carrier	r Name										
	US DOT#	/ State										
Comr	mercial / Non-Com	nercial	Not Provi	ded								
_												
Passenger I	Information							~				
EMS Transport						Sa	atety Equip Used					
Date of Death						_	Airbag I	Deployment Type				
Position In / On Vehicle		hicle					I	Ejected fr	om Vehicle Type			
									Injury Type			



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## **TECHNICAL MEMORANDUM**

DATE:	May 28, 2020
TO:	Mr. Tyler Gill, PE.
FROM:	Asma Ali, PhD, P.E., PTOE
RE:	I-81 Widening MM 136.6 to 141.8 Design-Build Project

### Introduction

This technical memorandum presents safety analysis results for the I-81 Widening Design-Build project in Roanoke County/City of Salem, VA. The Project limits extend 3,000 feet south of I-81 and Route 112 (Wildwood Road) interchange near mile marker 136.6 to 1,500 north of the I-81 and Route 419 (North Electric Road) interchange near mile marker 141.8. The study segment also includes the interchange of I-81 at Thompson Memorial Drive.

Currently, I-81 operates as a four-lane road with two lanes in the northbound (NB) direction and two lanes in the southbound (SB) direction. The I-81 widening project involves adding one mainline lane in each direction as well as widening the inside shoulder from 4-foot to 12-foot. The project build-out year is 2024 and design year is 2045.

Safety analysis of the I-81 mainline segment was performed to determine the impacts of the proposed design (3-lanes in each direction and a 12-foot inside shoulder) and an alternative concept (3-lanes in each direction and a 6-foot inside shoulder) for future crash mitigation. Three-year crash data, from January 1, 2017 through December 31, 2019, was downloaded from VDOT Traffic Engineering Department (TED) Online Crash Tableau. Table 1 presents a summary of crash trend by type, severity, lighting, surface conditions, and time of day.

### **Crash Data Summary**

As detailed in Table 1, a total of 326 crashes occurred on I-81 during the three-year period. Out of 326 crashes, 182 (56%) occurred on I-81 NB and 144 (44%) occurred on I-81 SB. The crash frequency increased by 10% from 2017 to 2018 and by 4% from 2018 to 2019.

The most prevalent crash type in both directions was rear-end crashes (139 out of 326 of 42.64%), followed by fixed object off-road (70 out of 326 or 21.5%), and sideswipe same direction crashes (55 out of 326 or 21.47%). Rear-end and sideswipe same direction crashes typically occur on freeways with closely spaced interchanges where vehicles are either in a continuous "stop and go" condition or engage in sudden lane changes at the merge and diverge areas. Speeding is reported as one of the potential causes for 59% (82 out of 139) rear-end crashes.

Out of the total 70 fixed object off-road crashes, 42 (60%) involved vehicles running off to the left side of the road. The proposed widening of the inside shoulder from 4-foot to 12-foot is expected to mitigate future fixed object off-road crashes by providing adequate distance to the errant vehicles to decelerate and stop on the shoulder. Of the 70 crashes, 35 (50%) occurred under wet surface conditions, 28 (40%) occurred during darkness, and 30 (42%) involved speeding. Adverse weather conditions and speeding appear to be the causal factors for the existing roadway departure crashes in both directions.

Out of 326 total crashes, three (3) crashes resulted in fatalities, of which two (2) occurred on I-81 NB and one (1) occurred on I-81 SB. On I-81 NB, both fatality related crashes involved fixed object off-road collisions and



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## **TECHNICAL MEMORANDUM**

the fatality crash on I-81 SB involved a rear-end crash. There were a total of 49 injury crashes which involved 65 injuries.

Thirty-three (33) percent of the total crashes occurred under wet surface conditions and 30% occurred during darkness when the roadway was not lighted. About 28% (90 out of 326) of the total crashes occurred during the PM peak period, from 3:00 PM to 6:00 PM, of which 73 were rear-end and sideswipe same direction crashes. Both rear-end and sideswipe same direction are considered congestion related crashes and typically occur during the peak periods. GIS crash maps by type and severity are provided in **Appendix A**.

		-	81 NB	ŀ	-81 SB	I-81 NB & SB	I-81 NB & SB
	Crash Type	No. of Crashes	% of Total Crashes	No. of Crashes	% of Total Crashes	Total Crashes	% of Total Crashes
'ear	2017	50	27.47%	33	22.92%	83	25.46%
sh)	2018	62	34.07%	53	36.81%	115	35.28%
Cra	2019	70	38.46%	58	40.28%	128	39.26%
	Total	182	100.00%	144	100.00%	326	100.00%
	Rear End	78	42.86%	61	42.36%	139	42.64%
	Fixed Object - Off Road	37	20.33%	33	22.92%	70	21.47%
ype	Sideswipe - Same Direction	37	20.33%	18	12.50%	55	16.87%
ЧЦ	Deer	16	8.79%	20	13.89%	36	11.04%
Cras	Angle	11	6.04%	6	4.17%	17	5.21%
Ŭ	Non-Collision	2	1.10%	3	2.08%	5	1.53%
	Other	1	0.55%	3	2.08%	4	1.23%
	Total	182	100.00%	144	100.00%	326	100.00%
~	Fatal Injury	2	1.10%	1	0.69%	3	0.92%
erit	Severe Injury	3	1.65%	0	0.00%	3	0.92%
e č	Visible Injury	21	11.54%	18	12.50%	39	11.96%
sh	Nonvisible Injury	4	2.20%	3	2.08%	7	2.15%
g	Property Damage Only	152	83.52%	122	84.72%	274	84.05%
	Total	182	100.00%	144	100.00%	326	100.00%
	Dawn	2	1.10%	3	2.08%	5	1.53%
å	Daylight	126	69.23%	87	60.42%	213	65.34%
hti	Dusk	1	0.55%	4	2.78%	5	1.53%
Lig	Darkness - Road Lighted	1	0.55%	1	0.69%	2	0.61%
	Darkness - Road Not Lighted	52	28.57%	49	34.03%	101	30.98%
	Total	182	100.00%	144	100.00%	326	100.00%
nd.	Dry	147	80.77%	104	72.22%	251	76.99%
e S	Wet	31	17.03%	35	24.31%	66	20.25%
rfac							
Sui	Snowy	4	2.20%	5	3.47%	9	2.76%
	Total	182	100.00%	144	100.00%	326	100.00%
	12:00 AM - 6:00 AM	23	12.64%	16	11.11%	39	11.96%
>	6:00 AM - 9:00 AM	18	9.89%	12	8.33%	30	9.20%
Dar	9:00 AM - 12:00 PM	17	9.34%	23	15.97%	40	12.27%
e of	12:00 PM - 3:00 PM	38	20.88%	25	17.36%	63	19.33%
Ĭ	3:00 PM - 6:00 PM	59	32.42%	31	21.53%	90	27.61%
-	6:00 PM - 9:00 PM	17	9.34%	17	11.81%	34	10.43%
	9:00 PM - 12:00 AM	10	5.49%	20	13.89%	30	9.20%
	Total	182	100.00%	144	100.00%	326	100.00%

### Table 1 – I-81 Crash Summary (2017 – 2019)



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## **TECHNICAL MEMORANDUM**

### Crash Rate

Table 2 presents a comparison of the total crash, injury crash and fatality crash rates for each direction with the statewide and districtwide rates. The total crash, injury crash and fatality rates were calculated as per 100 million vehicle miles (per 100 MVM).

On I-81 NB, the crash rate is 30% higher than the statewide rate and 60% higher than the district wide crash rate. The injury crash rate on I-81 NB is lower than the statewide rate but 23% higher than the district wide rate. The fatality crash rate on I-81 NB is over 100% higher than both statewide and district wide rates.

On I-81 SB, the crash rate on the study segment is 8% higher than the statewide rate and 35% higher than the districtwide rate. The injury crash rate on the study segment is lower than both statewide and districtwide rates. The fatality crash rate on I-81 SB is 2 to 3 times higher than the statewide and districtwide rates.

	Crash Rate	Injury Crash Rate	Fatality Crash Rate
	(Per 100 MVM)	(Per 100 MVM)	(Per 100 MVM)
I-81 NB	108.94	16.76	1.2
I-81 SB	90.2	13.15	0.63
Statewide Rate	83.45	21.43	0.01
Districtwide Rate (Salem)	66.26	13.55	0.34

### Table 2: Crash/Injury/Fatality Crash Rates

## Crash Mitigation

As stated earlier, the project proposes to add one lane in each direction and widen the inside shoulder. The proposed improvements were examined further using crash modification factors (CMFs) which can be used to determine the potential safety benefits of the proposed improvements. CMFs were chosen from the Virginia State Preferred CMF List. CMFs were applied to the following scenarios:

- Proposed Design 4- Lane to 6-Lane freeway and widening inside shoulder from 4-foot to 12-foot.
- Alternative Concept 4- Lane to 6-Lane freeway and widening inside shoulder from 4-foot to 6-foot.

Table 3 presents the applicable CMFs and their potential safety benefits.



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## **TECHNICAL MEMORANDUM**

	Widen from 4 Lanes to	Widen Paved Inside Shoulder from X Feet to				
	6 Lanes	Feet				
		CMF (KABC)	CMF (PDO)			
		e^0.0172*(x-y)	e^0.0153*(x-y)			
		From 4' to 12' shoulder	From 4' to 12' shoulder			
		CMF = 0.87	CMF = 0.88			
	CMF	From 4' to 6' shoulder	From 4' to 6' shoulder			
	0.7	CMF = 0.96	CMF = 0.97			
	Total Crashes = 326	KABC Crashes = 52	PDO Crashes = 274			
Roadway Design		Crash Reduction				
Proposed Design	98	7	32			
Alternative Concept	98	2	8			

### Table 3: Projected Reduction in Crashes

The CMFs indicates a potential 30% reduction in total crashes when a freeway is widened from four to six lanes. As in the proposed design, due to the widening of the inside paved shoulder from 4-foot to 12-foot, a potential reduction of 13% fatality and injury (KABC) crashes (CMF = 0.87), and 12% reduction in Property Damage Only (PDO) crashes (CMF = 0.88) is expected. As in the alternative concept, with the widening of the paved inside shoulder from 4-foot to 6-foot, a potential reduction of 4% KABC crashes (CMF = 0.96) and 3% reduction in PDO crashes (CMF = 0.7) is expected. In terms of crash reduction, the proposed design has a four times higher safety benefit over the alternative concept.

## Conclusion

A three-year (2017 to 2019) crash history was reviewed for the study segment of northbound and southbound I-81. A total of 326 crashes occurred during three years with 56% crashes in the northbound direction and 44% in the southbound direction. The most prevalent crash type was rear-end collisions followed by fixed object off-road crashes. The total crash and fatality crash rates in both directions are higher than the statewide and districtwide rates. The Virginia State preferred CMFs indicate four times higher safety benefits of the proposed design over the alternative concept. In the proposed design (widening the inside shoulder from 4-foot to 12-foot), the expected reduction of fatality and injury crashes is 13% and PDO crashes is 12%, whereas for the alternative concept (widening the inside shoulder from 4-foot to 6-foot), the expected crash reduction of fatality and injury crashes is 3%.

## APPENDIX A – GIS CRASH MAPS



I-81 Mainline & Ramps Crashes by Type and Severity MP 136.60 to MP 137.48 2017-2019 Sheet 1 of 7





### **Crash Severity**





Shoulders requiring Design Exception



I-81 Mainline & Ramps **Crashes by Type and Severity** MP 137.81 to MP 139.00 2017-2019 Sheet 3 of 7





### **Crash Severity**





I-81 Mainline & Ramps Crashes by Type and Severity MP 139.00 to MP 139.96 2017-2019 Sheet 4 of 7





### **Crash Severity**





I-81 Mainline & Ramps Crashes by Type and Severity MP 139.96 to MP 140.79 2017-2019 Sheet 5 of 7









I-81 Mainline & Ramps Crashes by Type and Severity MP 140.79 to MP 141.29 2017-2019 Sheet 6 of 7





### **Crash Severity**







I-81 Mainline & Ramps Crashes by Type and Severity MP 141.29 to MP 141.80 2017-2019 Sheet 7 of 7





Shoulders Requiring Design Exception


# COMMONWEALTH of VIRGINIA

DEPARTMENT OF TRANSPORTATION 731 HARRISON AVENUE SALEM, VIRGINIA 24153

Stephen Brich, P.E. COMMISSIONER

June 15, 2020

To: Craig Moore Design Engineer

From: Carol J.L. Moneymaker Planning Specialist

Carol J. L. Moneymaker

Subject:	Traffic Data for Scoping Report and Design Purposes
Route:	I-81
Project:	0081-080-946
UPC:	116203
Location:	Roanoke County
From:	Overpass of Route 641 (MP 136.85)
To:	0.3 MI north of Route 419 Overpass (MP 141.91)

Listed below is the requested data for the project referenced above. For mainline counts, text in *italics* indicates data derived from counts taken in 2017; all other mainline data is calculated from counts taken in 2019. All ramp data is calculated from counts taken in 2017.

I-81 Between Exits 132 and 137	Northbound	Southbound	Combined
Current ADT (2019)	27,296	23,149	50,445
Design Year ADT (2043):	31,882	27,038	58,920
Design Hourly Volume (DHV):	N/A	N/A	5,503
Directional DHV (Trucks):	N/A	N/A	901
Existing AM Peak Hour Traffic:	1,745	1,603	3,342
Existing AM Peak Hour Period	7:15 - 8:15	7:30 - 8:30	7:15 - 8:15
Existing PM Peak Hour Traffic:	2,235	2,303	4,507
Existing PM Peak Hour Period:	4:15 - 5:15	4:30 - 5:30	4:30 - 5:30
AM Directional Distribution Factor*	1,745 (52%)	1,597 (48%)	3,342
PM Directional Distribution Factor*	2,204 (49%)	2,303 (51%)	4,507
Peak Hour Factor:	0.98	0.93	0.97

Truck Percentage (Northbound)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		4.0% 4.1%	18.3% 25.8%	22.3% 29.9%
Truck Percentage (Southbound)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		2.5% 4.1%	14.3% 22.4%	16.8% 26.5%
Truck Percentage (Combined)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		3.0% 4.1%	16.4% 24.1%	19.4% 28.2%
I-81 Between Exits 137 and 140	Northbound	Sout	hbound	Combined
Current ADT (2019)	31,659	31,7	65	63,424
Design Year ADT (2043):	41,537	41,6	76	83,212
Design Hourly Volume (DHV):	N/A	N/A		6,823
Directional DHV (Trucks):	N/A	N/A		922
Existing AM Peak Hour Traffic:	2,100	1,98	1	4,081
Existing AM Peak Hour Period	7:30 - 8:30	7:30	- 8:30	7:30 - 8:30
Existing PM Peak Hour Traffic:	2,596	2,86	9	5,458
Existing PM Peak Hour Period:	4:15 - 5:15	4:45	- 5:45	4:45 - 5:45
AM Directional Distribution Factor*	2,100 (51%)	1,98	1 (49%)	4,081
PM Directional Distribution Factor*	2,589 (47%)	2,86	9 (53%)	5,458
Peak Hour Factor:	0.98	0.94		0.97
Truck Percentage (Northbound)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		3.2% 4.0%	16.8% 23.2%	19.9% 27.2%
Truck Percentage (Southbound)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.0% 1.7%	0.7% 1.2%	12.8% 20.0%	14.4% 22.9%
Truck Percentage (Combined)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		2.2% 3.5%	14.6% 21.5%	16.8% 25.0%

I-81 Between Exits 140 and 141	Northbound	Sout	hbound	Combined
Current ADT (2019)	33,436	33,5	87	67,023
Design Year ADT (2043):	43,066	43,2	60	86,326
Design Hourly Volume (DHV):	N/A	N/A		7,856
Directional DHV (Trucks):	N/A	N/A		1,064
Existing AM Peak Hour Traffic:	2,626	2,36	1	5,005
Existing AM Peak Hour Period	7:30 - 8:30	7:15	- 8:15	7:15 - 8:15
Existing PM Peak Hour Traffic:	2,703	2,76	2	5,436
Existing PM Peak Hour Period:	4:00 - 5:00	4:30	- 5:30	4:15 - 5:15
AM Directional Distribution Factor*	2,644 (53%)	2,36	1 (47%)	5,005
PM Directional Distribution Factor*	2,679 (49%)	2,75	7 (51%)	5,436
Peak Hour Factor:	0.97	0.96		0.97
Truck Percentage (Northbound)	Class 4-5	<u>Class 6-7</u>	Class 8-13	<u>Total</u>
% Peak % Daily	2.1% 2.0%	0.8% 1.2%	18.1% 24.1%	21.0% 27.3%
Truck Percentage (Southbound)	<u>Class 4-5</u>	Class 6-7	<u>Class 8-13</u>	Total
% Peak % Daily	1.4% 2.1%	1.4% 1.5%	14.4% 22.1%	17.3% 25.7%
Truck Percentage (Combined)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	Total
% Peak % Daily	1.8% 2.0%	1.2% 1.3%	16.3% 23.1%	19.2% 26.5%

I-81 Between Exits 141 and 143	Northbound	Sout	hbound	Combined
Current ADT (2019)	36,787	36,3	68	73,155
Design Year ADT (2043):	47,382	46,8	42	94,224
Design Hourly Volume (DHV):	N/A	N/A		8,603
Directional DHV (Trucks):	N/A	N/A		1,056
Existing AM Peak Hour Traffic:	3,088	2,78	б	5,874
Existing AM Peak Hour Period	7:15 - 8:15	7:15	- 8:15	7:15 - 8:15
Existing PM Peak Hour Traffic:	3,068	3,07	4	6,142
Existing PM Peak Hour Period:	4:15 - 5:15	4:15	- 5:15	4:15 - 5:15
AM Directional Distribution Factor*	3,088 (53%)	2,78	6 (47%)	5,874
PM Directional Distribution Factor*	3,068 (50%)	3,07	4 (50%)	6,142
Peak Hour Factor:	0.96	0.94		0.97
Truck Percentage (Northbound)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	Total
% Peak % Daily	1.5% 1.8%	0.7% 1.2%	14.7% 21.9%	16.9% 24.9%
Truck Percentage (Southbound)	<u>Class 4-5</u>	Class 6-7	<u>Class 8-13</u>	Total
% Peak % Daily	1.6% 2.0%	0.8% 1.2%	14.2% 20.4%	16.7% 23.6%
Truck Percentage (Combined)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	Total
% Peak % Daily	1.6% 1.9%	0.8% 1.2%	14.5% 21.1%	16.8% 24.2%

#### Ramp 1 - Exit 137: I-81 NB Exit Ramp to Route 112 (Wildwood Road)

Current ADT (2017):	2,590		
Design Year ADT (2043):	3,061		
Design Hourly Volume (DHV):	301		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.0% 3.6%	0.0% 4.9%	0.0% 8.5%
Existing Peak Hour Traffic:	A.M.: 171 P.M.: 213	(7:00 AM - 8 (4:30 PM - 5:	:00 AM) :30 PM)
Peak Hour Factor:	0.92		

#### Ramp 2 - Exit 137: I-81 NB Entrance Ramp from Route 112 (Wildwood Road)

Current ADT (2017):	7,232		
Design Year ADT (2043):	8,548		
Design Hourly Volume (DHV):	676		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.6% 4.3%	0.9% 5.9%	1.5% 10.2%
Existing Peak Hour Traffic:	A.M.: 587 P.M.: 645	(7:15 AM - 8: (4:30 PM - 5:	15 AM) 30 PM)
Peak Hour Factor:	0.92		

#### Ramp 3 - Exit 137: I-81 SB Exit Ramp to Route 112 (Wildwood Road)

Current ADT (2017):	6,729		
Design Year ADT (2043):	7,954		
Design Hourly Volume (DHV):	764		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	Total
% Peak % Daily	0.7% 4.7%	1.0% 6.0%	1.7% 10.8%
Existing Peak Hour Traffic:	A.M.: 492 P.M.: 679	(7:30 AM - 8 (4:45 PM - 5:	:30 AM) :45 PM)
Peak Hour Factor:	0.92		

#### Ramp 4 - Exit 137: I-81 SB Entrance Ramp from Route 112 (Wildwood Road)

Current ADT (2017):	2,349		
Design Year ADT (2043):	2,777		
Design Hourly Volume (DHV):	243		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	<u>Total</u>
% Peak % Daily	0.3% 4.1%	0.8% 4.2%	1.0% 8.2%
Existing Peak Hour Traffic:	A.M.: 189 P.M.: 191	(7:15 AM - 8 (4:45 PM - 5	8:15 AM) :45 PM)
Peak Hour Factor:	0.90		

#### Ramp 5 - Exit 140: I-81 NB Exit Ramp to Route 311 (Thompson Memorial Drive)

Current ADT (2017):	2,608		
Design Year ADT (2043):	3,489		
Design Hourly Volume (DHV):	435		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.0% 4.4%	0.0% 2.0%	1.0% 6.4%
Existing Peak Hour Traffic:	A.M.: 245 P.M.: 243	(7:00 AM - 8: (5:00 PM - 6:0	00 AM) 00 PM)
Peak Hour Factor:	0.89		

#### Ramp 6 - I-81 NB Entrance Ramp from Route 311 (Thompson Memorial Drive)

Current ADT (2017):	4,303		
Design Year ADT (2043):	5,757		
Design Hourly Volume (DHV):	646		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	<u>Total</u>
% Peak % Daily	0.3% 2.6%	0.1% 1.0%	0.4% 3.6%
Existing Peak Hour Traffic:	A.M.: 410 P.M.: 480	(7:30 AM - 8 (5:30 PM - 6	8:30 AM) 5:30 PM)
Peak Hour Factor:	0.85		

Ramp 7 - I-81 SB Exit Ram	to Route 311 (Th	hompson Memorial Drive)

Current ADT (2017):	4,291		
Design Year ADT (2043):	5,741		
Design Hourly Volume (DHV):	640		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.1% 2.3%	0.0% 1.0%	0.1% 3.3%
Existing Peak Hour Traffic:	A.M.: 389 P.M.: 442	(7:30 AM - 8 (5:45 PM - 6	:30 AM) :45 PM)
Peak Hour Factor:	0.80		

# Ramp 8 - I-81 SB Entrance Ramp from Route 311 (Thompson Memorial Drive)

Current ADT (2017):	2,412		
Design Year ADT (2043):	3,227		
Design Hourly Volume (DHV):	344		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	Total
% Peak % Daily	0.4% 3.9%	0.7% 2.8%	1.1% 6.6%
Existing Peak Hour Traffic:	A.M.: 198 P.M.: 271	(7:15 AM - 8: (4:30 PM - 5:	15 AM) 30 PM)
Peak Hour Factor:	0.90		

#### Ramp 9 - I-81 NB Exit Ramp to Route 419 (Electric Road)

Current ADT (2017):	2,725		
Design Year ADT (2043):	3,645		
Design Hourly Volume (DHV):	370		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	<u>Total</u>
% Peak	1.5%	0.0%	1.5%
% Daily	5.3%	4.8%	10.0%
Existing Peak Hour Traffic:	A.M.: 231	(7:30 AM - 8:	30 AM)
	P.M.: 270	(4:30 PM - 5:	30 PM)
Peak Hour Factor:	0.86		

# Ramp 10 - I-81 NB Entrance Ramp from Route 419 (Electric Road)

Current ADT (2017)	6 214		
Current 71D 1 (2017).	0,214		
Design Year ADT (2043):	8,314		
Design Hourly Volume (DHV):	826		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	Total
% Peak	0.3%	0.2%	0.6%
% Daily	3.1%	1.9%	5.0%
Existing Peak Hour Traffic:	A.M.: 582	(7:15 AM - 8	:15 AM)
	P.M.: 612	(4:30 PM - 5:	30 PM)
Peak Hour Factor:	0.85		

#### Ramp 11 - I-81 SB Exit Ramp to Route 419 (Electric Road)

Current ADT (2017):	6,262		
Design Year ADT (2043):	8,380		
Design Hourly Volume (DHV):	959		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.6% 3.2%	0.4% 1.8%	1.0% 5.0%
Existing Peak Hour Traffic:	A.M.: 629 P.M.: 590	(7:15 AM - 8: (4:30 PM - 5:3	15 AM) 30 PM)
Peak Hour Factor:	0.85		

#### Ramp 12 - I-81 SB Entrance Ramp from Route 419 (Electric Road)

Current ADT (2017):	2,902		
Design Year ADT (2043):	3,882		
Design Hourly Volume (DHV):	355		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.4% 4.6%	0.7% 4.6%	1.1% 9.2%
Existing Peak Hour Traffic:	A.M.: 239 P.M.: 277	(7:15 AM - 8 (4:45 PM - 5	8:15 AM) 5:45 PM)
Peak Hour Factor:	0.94		

Route 112 (Wildwood Road) from NCL Sal	em to I-81			
Current ADT (2019):	20,360			
Design Year ADT (2043):	24,406			
Design Hourly Volume (DHV):	2,060			
Directional DHV (Trucks):	17			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	Cannot be cald 1.2%	culated with ava 0.1%	ailable count da 0.2%	ata. 1.5%
Directional Distribution Factor:	A.M.: 1,804 P.M.: 1,801	N: 870 (48%) N: 828 (46%)	S: 934 (52%) S: 973 (54%)	
Existing Peak Hour Traffic:	A.M.: 1,804 P.M.: 1,801	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
Peak Hour Factor:	0.90			

# Route 112 (Wildwood Road) from I-81 to FR 70

Current ADT (2019):	908			
Design Year ADT (2043):	1,017			
Design Hourly Volume (DHV):	127			
Directional DHV (Trucks):	2			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.5% 1.4%	0.5% 0.6%	0.0% 0.1%	2.0% 2.1%
Directional Distribution Factor:	A.M.: 82 P.M.: 101	N: 18 (22%) N: 63 (62%)	S: 64 (78%) S: 38 (38%)	
Existing Peak Hour Traffic:	A.M.: 82 P.M.: 101	(7:15 AM - 8: (5:15 PM - 6:2	15 AM) 15 PM)	
Peak Hour Factor:	0.90			

1100051	(A and ama	Ctract	from W	Completon	A vianua ta	Doute 610	Wildwood Dood)
UKOUJI	Academy	/ Street,	110111 w.	Carronon	Avenue to	Roule 019	(WIIGWOOD KOad)

Current ADT (2019):	1,950			
Design Year ADT (2043):	2,225			
Design Hourly Volume (DHV):	240			
Directional DHV (Trucks):	2			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	Class 8-13	<u>Total</u>
% Peak % Daily	0.5% 1.2%	0.0% 0.1%	0.0% 0.1%	0.5% 1.4%
Directional Distribution Factor:	A.M.: 218 P.M.: 186	N: 70 (32%) N: 113 (61%)	S: 148 (68%) S: 73 (39%)	
Existing Peak Hour Traffic:	A.M.: 218 P.M.: 186	(7:15 AM - 8: (5:00 PM - 6:0	15 AM) 00 PM)	
Peak Hour Factor:	0.88			

# Route 619 (Wildwood Road) from Route 733 to UR 8051 (Academy Street)

Current ADT (2019):	1,190			
Design Year ADT (2043):	1,333			
Design Hourly Volume (DHV):	154			
Directional DHV (Trucks):	1			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.4% 0.9%	0.4% 0.4%	0.7% 0.2%	1.4% 1.5%
Directional Distribution Factor:	A.M.: 112 P.M.: 140	E: 82 (73%) E: 63 (45%)	W: 30 (27%) W: 77 (55%)	
Existing Peak Hour Traffic:	A.M.: 112 P.M.: 140	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
Peak Hour Factor:	0.78			

Route 311 from Rose Ln to NCL Salem				
Current ADT (2019):	13,707			
Design Year ADT (2043):	15,352			
Design Hourly Volume (DHV):	1,640			
Directional DHV (Trucks):	17			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak	0.7%	0.1%	0.5%	1.2%
% Daily	1.0%	0.3%	0.7%	2.0%
Directional Distribution Factor:	A.M.: 1,462 P.M.: 1,580	N: 604 (41%) N: 875 (55%)	S: 858 (59%) S: 705 (45%)	
Existing Peak Hour Traffic:	A.M.: 1,462 P.M.: 1,580	(7:15 AM - 8: (4:30 PM - 5:3	15 AM) 30 PM)	
Peak Hour Factor:	0.94			

# Route 311 from NCL Salem to Route 419

Current ADT (2019):	5,934			
Design Year ADT (2043):	6,646			
Design Hourly Volume (DHV):	750			
Directional DHV (Trucks):	8			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.7% 1.0%	0.1% 0.3%	0.5% 0.7%	1.2% 2.0%
Directional Distribution Factor:	A.M.: 486 P.M.: 603	N: 162 (33%) N: 349 (58%)	S: 324 (67%) S: 254 (42%)	
Existing Peak Hour Traffic:	A.M.: 486 P.M.: 603	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
Peak Hour Factor:	0.94			

#### Route 419 from NCL Salem to I-81

	Current ADT (2019):	12,773			
	Design Year ADT (2043):	14,306			
	Design Hourly Volume (DHV):	1,702			
	Directional DHV (Trucks):	45			
	Truck Percentage:	Class 4-5	Class 6-7	<u>Class 8-13</u>	Total
	% Peak % Daily	0.4% 1.4%	0.3% 0.8%	1.3% 2.6%	2.0% 4.8%
	Directional Distribution Factor:	A.M.: 1,428 P.M.: 1,511	N: 640 (45%) N: 827 (55%)	S: 788 (55%) S: 684 (45%)	
	Existing Peak Hour Traffic:	A.M.: 1,428 P.M.: 1,511	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
	Peak Hour Factor:	0.95			
Route 4	419 from I-81 to Route 311				

Current ADT (2019):	9,353			
Design Year ADT (2043):	10,475			
Design Hourly Volume (DHV):	983			
Directional DHV (Trucks):	30			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.4% 1.4%	0.3% 0.8%	1.3% 2.6%	2.0% 4.8%
Directional Distribution Factor:	A.M.: 958 P.M.: 1,078	N: 271 (28%) N: 672 (62%)	S: 687 (72%) S: 406 (38%)	
Existing Peak Hour Traffic:	A.M.: 958 P.M.: 1,078	(7:00 AM - 8: (4:30 PM - 5:3	00 AM) 30 PM)	
Peak Hour Factor:	0.95			

If you have questions or need additional information, please contact me at (540) 387-5228.

cc:	Alex Vandyke	Joyce Barkley
	Bruce Penner	Jason Henry
	L.J. Muchenje	Anne Booker
	Alex Price	Ray Varney
	Michael Gray	

Appendix E

# 1.7 Project Assumptions & Understandings

Certain aspects surrounding the project affect the scope of work detailed in Section 2. These assumptions and current understandings are outlined below.

- VDOT has NO desire to replace or reconstruct any bridges passing over I-81; Red Lane overpass and North Electric Road (419) may require some type of waiver or exception along I-81 to meet this goal
- VDOT is considering replacing affected bridges along I-81; VDOT desires the RS&H Team evaluate this decision and make recommendations regarding widening/reconstruction verses



# Memorandum I-81 MM 136.6 to 141.8 D-B UPC 116203

To: File
From: Tyler Gill
CC: VDOT Project Team RS&H Design Team
Date: May 15, 2020

**Re:** RFP Bridge Strategy Determination Meeting Notes/Design Direction

This memo documents the decisions/design direction that was agreed to at the Bridge Strategy Determination Discussion on 2020-05-08. The meeting was held virtually via MS Teams and included VDOT Salem District, VDOT CO, FHWA, and RS&H Design Team staff. An attendee list is included as an attachment.

The design team will incorporate the directives noted below in the development of the RFP plans.

#### NB (B683) & SB (B688) I-81 Over Rt. 112 (Wildwood Rd.)

The RFP bridge strategy for this location:

- REPLACE both bridge structures (as recommended by PRIME AE Group (PRIME AE) and concurred with by VDOT and FHWA)
- Adjust vertical grades to achieve a 16' 6" minimum vertical clearance (as recommended by PRIME AE and concurred with by VDOT and FHWA)

- Accommodate an additional single lane widening on I-81 (in addition to this project; 4 total thru lanes) with a vertical clearance of 16' 6" (Ch. 6 S&B Manual)
- Anticipate DB Offeror revising bridge span arrangement Design Team to consider single span and 2 span alternatives with closed abutments for RFP. Update roadway and bridge plans and estimate as needed.
- Design Team to account for potential Rt. 112 widening (additional single lane in each direction) in span arrangement (Ch. 17 S&B Manual)
  - Develop an ultimate Rt. 112 typical section and 'Structural Obstruction Zone' (SOZ) (similar to I-81 Exit 114 DB project) to gain concurrence from team members
  - VDOT has ability to waive this requirement
- Technical Requirement language to require replacement, minimum vertical clearance, and accommodation of Rt. 112 widening. SOZ sheet can be included.

- Technical Requirement language to not preclude grade adjustments/improvements to Rt. 112
- Design Team to determine if lighting underneath bridge will be required
- Include pier protection on Rt. 112 if required
- VDOT to explore City of Salem's desire for aesthetic treatment. RFP plans and estimate to be updated accordingly.

Other items:

- RFQ Replacement Concept (submitted 2020-04-24) is sufficient for RFQ Plans
- FHWA request Add cost of fatigue prone details
- FHWA request No need to update LCCA for replacement option

#### NB (B684) & SB (B685) I-81 Over Rt. 635 (Goodwin Ave.)

The RFP bridge strategy for this location:

- REPLACE both bridge structures (as recommended by PRIME AE and concurred with by VDOT and FHWA)
- Design Team to explore three vertical clearance options:
  - Adjust vertical grades to achieve a 16' 6" minimum vertical clearance
  - Adjust vertical grade to achieve 15' 8" minimum vertical clearance (to match existing NB bridge clearance)
  - Adjust vertical grade to achieve a 14' 6" minimum vertical clearance

- VDOT willing to entertain vertical clearance below 16' 6"
  - Design Waiver required for vertical clearance above 14' 6" but below 16' 6"
  - Design Team to provide cost estimates for all vertical clearance options for comparison/assisting in bridge clearance determination
- VDOT to explore including an RFP bid option for DB Offerors to provide a 16' 6" vertical clearance
- Accommodate an additional single lane widening on I-81 (in addition to this project; 4 total thru lanes) meeting the vertical clearance selected
- Pursue waiver for potential Rt. 635 widening accommodation (Ch. 17 S&B Manual)
- VDOT to vet potential 'no bridge' alternative (cul-de-sac Rt. 635) internally and with local
  officials

- If feasible Design Team to explore existing condition analysis (geometric and traffic) to determine if upgrades /improvement will be needed to other routes
- If feasible VDOT needs to begin coordination with localities/public as DB Offerors could view securing approvals as a high risk item
- Design Team to explore high level existing conditions analysis (geometric and traffic) to determine if upgrades/improvements will be needed to 'detour' routes during construction.
- Technical Requirement language to require replacement and minimum vertical clearance. If permanent closure of Rt. 635 not feasible, reinforce with TR language. May need to add TR section communicating concurrent work restrictions on Rt. 635 and Rt. 619 bridges.
- Include pier protection on Rt. 635 if required
- VDOT to explore City of Salem's desire for aesthetic treatment. RFP plans and estimate to be updated accordingly.

Other Items:

- RFQ Replacement Concept (submitted 2020-04-24) is sufficient for RFQ Plans
- FHWA will not approve design exception for vertical clearance less than 14' 6"
- Rt. 635 classified as Urban Minor Collector which requires minimum vertical clearance of 14' 6"
- Achieving 14' 6" and/or 15' 8" vertical clearance can largely be obtained via lowering Rt. 635

#### NB (B687) & SB (B686) I-81 Over Rt. 619 (Wildwood Rd./Academy St.)

The RFP bridge strategy for this location:

- REPLACE both bridge structures (as recommended by PRIME AE and concurred with by VDOT and FHWA)
- Provide a 16' 6" minimum vertical clearance (as recommended by PRIME AE and concurred with by VDOT and FHWA)

- Accommodate an additional single lane widening on I-81 (in addition to this project; 4 total thru lanes) with a vertical clearance of 16' 6" (Ch. 6 S&B Manual)
- Anticipate DB Offeror revising bridge span arrangement Design Team to consider single span alternative with closed abutments for RFP. Update roadway and bridge plans and estimate as needed.
- Design Team to account for potential Rt. 619 widening (additional single lane in each direction) in span arrangement (Ch. 17 S&B Manual)

- Develop an ultimate Rt. 619 typical section and 'Structural Obstruction Zone' (SOZ) (similar to I-81 Exit 114 DB project) to gain concurrence from team members
- VDOT has ability to waive this requirement
- Technical Requirement language to require replacement, minimum vertical clearance, accommodation of Rt. 619 widening, and potential minimum width between structures. SOZ sheet can be included. May need to add TR section communicating concurrent work restrictions on Rt. 635 and Rt. 619 bridges as well as allowable closures/minimum MOT typical sections.
- Include pier protection on Rt. 619 if required
- VDOT to explore City of Salem's desire for aesthetic treatment. RFP plans and estimate to be updated accordingly.

Other Items:

- RFQ Replacement Concept (submitted 2020-04-24) is sufficient for RFQ Plans
- Potential location for DB Offeror incentive

#### Rt. 705 (Red Ln.) (B682) Over NB & SB I-81

The RFP bridge strategy for this location:

- Existing bridge structure to remain
- Proposed design to provide a 16' 6" minimum vertical clearance
- Provide pier protection

Additional elements for RFP plan and document development:

• VDOT to investigate if required repair work is needed to this structure. If so, include in Technical Requirements.

Other Items:

• Substructure condition rated a 5.

#### NB (B678) & SB (B677) I-81 Over Rt. 311 (Thompson Memorial Hwy.)

The RFP bridge strategy for this location:

- WIDEN both bridge structures (as recommended by PRIME AE and concurred with by VDOT and FHWA)
- Provide a 16' 6" minimum vertical clearance (as recommended by PRIME AE and concurred with by VDOT and FHWA)

#### Appendix F

Additional elements for RFP plan and document development:

- Design Team to analyze proposed inside shoulder width
  - o Affects distance between structures
  - o If less than 12' a design waiver is required
- Design Team to determine if lighting underneath bridge will be required
- Technical Requirement language to require widening (at a minimum), minimum vertical clearance, and potential minimum width between structures. Require any widening aesthetics match existing.
- Include pier protection on Rt. 311 if required
- VDOT to explore City of Salem's desire for aesthetic treatment. RFP plans and estimate to be updated accordingly.

#### Other Items:

- RFQ Widening Concept (submitted 2020-04-24) is sufficient for RFQ Plans
- FHWA request Using ultrasonic peening (or other methods) address all fatigue prone details (extend fatigue life of the details to service life of the bridge), include cost for fatigue peening
- Future maintenance project being developed, to include painting and aesthetic features. Anticipate complete prior to 116203 begin construction.

#### NB & SB I-81 Over Rt. 630 (Kessler Mill Rd.)

The RFP bridge strategy for this location:

• Existing bridge structure to remain

Additional elements for RFP plan and document development:

• If preference in method/how bridge will be restriped, include in Technical Requirements.

#### Rt. 419 (Electric Rd.) (B681) Over NB & SB I-81

The RFP bridge strategy for this location:

- Existing bridge structure to remain
- Proposed design to provide a 16' 6" minimum vertical clearance
- Provide pier protection

- VDOT to investigate if required repair work is needed to this structure. If so, include in Technical Requirements.
- Proposed inside and outside shoulders require Design Exceptions for width
  - Design Team to develop DE document prior to RFP. Coordinate with VDOT and FHWA (S. Clausen)
  - Design Team to review crash data at overpass location to determine if specific mitigation measures may be warranted

#### Other Items:

- Minimum shoulder widths: NB 6.4' (inside); SB 4.04' (inside)
- Pier Protection barrier height will be 54"



# Memorandum I-81 MM 136.6 to 141.8 D-B UPC 116203

To: Craig Moore (VDOT)

From: Tyler Gill

CC: Alex Price, Alex Vandyke, Tony Dodson (VDOT)

John Vandergriff, Kyle Faby (RS&H)

Date: June 3, 2020

**Re:** Summary of Concepts and Preferred Concept Recommendation

This memo serves to capture key attributes of the considered concepts for UPC 116203. The memo documents key elements of each concept as presented as part of a the RFQ strategy work session with VDOT. The RS&H Team explored constructability, SOC, construction schedule, ROW impacts, MOT, bridge replacement/widen strategy, bifurcation effects, median width, and other significant factors during the concept evaluations.

#### **Project Goals**

The primary project goals which guide the design concepts are:

- Increase operational capacity of mainline I-81 by one lane in each direction
- Consider the complexity of construction of each concept for both the traveling public and the builder
- Apply common sense geometric and cross-sectional design features to maintain or improve the safety of mainline I-81

#### Widening Strategies

Initially RS&H evaluated two basic alternatives for the I-81 Widening MM 136.6 to 141.8 DB Project – inside widening and outside widening. In simplest form, the inside widening concept proposes to add a third travel lane to both northbound and southbound I-81 within the existing median. The outside widening concept proposes the same lane addition, locating the additional lane to the outside in each direction. Following the initial examination of these concepts, RS&H determined a combination of inside and outside widening strategies should also be explored.

Four preliminary widening concepts were advanced to an RFQ Strategy Work Session for discussion with the project team. Each are summarized below.

- 1. Inside-Inside Concept (II) widening to median in each direction
- 2. Outside-Outside Concept (OO) widening to outside in each direction
- 3. NB Inside-SB Outside Concept (SBO) widening northbound to median (west) and southbound to outside (west)

4. NB Outside-SB Inside Concept (NBO) – widening northbound to outside (east) and southbound to median (east)

#### 1 Inside-Inside Concept (II)

Proposed Inside-Inside concept considers the viability of widening I-81 in each direction within the existing median with minimal roadway improvements to the outside lanes. As shown in Figure 1.1 below, the II Concept adds the proposed lane by extending the negative cross slope and holds the existing crown (generally in the center of the NB/SB lanes) for the majority of the project reducing the need to perform pavement wedging. ROW impacts associated with concept II widening are minimized but still exist for various elements like SWM, noise walls, and other elements. Interchange improvements to the existing ramps and loops are less significant and consist mainly of tie in work.





The variable width existing bifurcated median constrains this concept. Approximately one half of the widening project necessitates sub-standard inside shoulder widths which requires associated design waivers/exceptions. The existing bifurcation through the corridor greatly increases construction complexity, requires special design barrier/retaining structures, and constructability is further complicated should bridge replacement be selected.

As typical in Figure 1.1. certain proposed features of concept II prove sub-optimal for the length of the project:

- Cross sectional features do not conform to VDOT/AASHTO standards (2.3 miles of substandard shoulder width)
- 2.3 miles of median barrier, of which about 2 miles are special design (soil retaining) barriers
- Complex storm sewer systems and complex interface with existing transverse culverts
- Limited workspace and difficult hauling routes within the median area due to wall construction / bifurcation
- Complicated erosion and sediment control due to bifurcation/proposed walls
- Complicated construction access due to bifurcation / proposed walls
- I-81 has large % truck traffic and bifurcated retaining walls pose safety challenges for errant trucks breaking thru rigid barriers on the high side of bifurcation. The proximity of the lower (elevation) opposing travel way poses increased probability for severe accidents.

Additionally, constructability varies greatly depending on the selected bridge strategy - widening or replacement. A widening strategy for the existing bridge structures to the inside is preferred with the II Concept as the median width can accommodate the additional lanes with reduced shoulders at all

locations. Bridge construction can generally occur by shifting traffic onto the existing shoulders to accomplish this. SOC/MOT for bridge replacement Concept II is likely very difficult due to the bifurcation, existing median width, and the requirement to maintain two lanes of traffic in each direction at all times across the existing bridge. Complicated bridge sequencing includes potential splitting of traffic (same direction), multiple bridge cuts, and additional construction phases.

#### 2 Outside-Outside Concept (OO)

Proposed Outside-Outside (OO) Concept widens I-81 in each direction to the outside of the existing lanes largely preserving the existing graded median and generally adhering to VDOT/AASHTO cross sectional element standards. Like the II concept, Concept OO generally maintains the existing crown location for the majority of the project limits, reducing pavement wedging. Additional positive attributes include simplified mainline construction, preservation of existing median widths, and simplified drainage conveyance.

Certain suboptimal attributes of Concept OO are readily apparent like:

- This is the most impactful concept
- Substantial impacts to the eastern and more populated portion of the alignment requiring right of way acquisitions and residential relocations
- Significant impacts to the existing interchanges on the eastern portion of the alignment where this portion of the topography is lower in elevation requiring steep ramp grades
- A more complex sequence of construction resulting from the impacts to the interchanges both on the east (more populated) and west (less populated) side of mainline I-81
- Frontage road relocations will be necessary and retaining walls in interchange and frontage roads areas are likely

Constructability varies depending on the selected bridge strategy, widening or replacement. Widening of the existing bridge structures to the outside can be accommodated by shifting traffic onto the existing inside shoulders.

While bridge replacement would not be as complex as other concepts, bridge replacement may require the need to 'over build' to the outside to maintain two lanes of traffic in each direction during construction. Wider bridge decks will require further raising of the I-81 grade at bridge locations with substandard existing vertical clearance.

#### 3 NB Inside-SB Outside Concept (SBO)

The SBO concept is a hybrid of the II and OO Concepts wherein the SBO Concept adds an additional thru lane to the inside of NB I-81 (median) and outside of SB I-81 (west) as shown in Figure 3.1 below.



Figure 3.1

By comparison, the roadway improvements associated with the SBO and II Concepts are similar from the Kessler Mill Rd. bridge to the northern project termini. The SBO Concept optimizes several aspects associated with I-81 widening such as:

- SBO provides a more reasonable level of construction complexity, as the entire median may be used for construction of only a single lane
- Concept SBO reduces the adverse constraints resulting from the existing bifurcation by accomplishing half of the widening (SB direction) to the outside
- This concept alleviates most constraints for the inside shoulders allowing for proper shoulder widths that meet VDOT/AASHTO standards. (existing overpasses are likely exception)
- The same as II and OO, the location of the existing crown is maintained where possible within the project.
- The more difficult portions of the interchanges to reconstruct are largely on the east side of the corridor and concept SBO largely avoids reconstruction of the east side (outside) northbound interchanges.
- ROW impacts for roadway widening are anticipated along SB I-81, where residential density is lower.
- As the project topography slopes west to east, SBO affords the maximum space for the location of SWM facilities and ESC measures on the downstream side of the project.
- Likely noise walls will be placed adjacent to NB I-81 and while there could still be ROW
  impacts associated with noise walls, the SBO concept minimizes property impacts by shifting
  the roadway improvements to the inside (median) along the northbound direction.

Permanent median barrier will still be needed for concept SBO; however, the amount of special design barrier/retaining structure will be dramatically reduced.

As with the other concepts, SOC/MOT is dependent on the bridge improvement strategy. If bridge replacement is selected for multiple bridge pairs, SBO allows for phased bridge construction without the need for 'over build', interchange ramp/loop closures, and large numbers of traffic shifts. The bridges are able to be replaced by constructing a portion of one structure in the median and using it as a reversible 'diversion' to sequence the remaining structural elements. This approach reduces bridge cuts and allows for one structure to potentially be constructed completely outside of traffic.

#### 4 NB Outside-SB Inside Concept (NBO)

NBO is the inverse of the SBO concept, with roadway widening occurring outside of NB I-81 (east) and the inside of SB I-81 (median). The same features of maximized inside shoulders that generally conform to VDOT/AASHTO standards, improved constructability, minimized effects of the existing bifurcation, and maintaining the existing crown that are realized with SBO, apply to NBO, as well. However, the sub-optimal impacts are most significant on the east side of the northbound lanes namely:

 Concept NBO involves a scenario where roadway widening, SWM devices, and potential noise walls all are east of I-81. Locating these major project components adjacent to the City of Salem and a highly residential area will result in a more involved ROW process and expense

- This alternative is the second most impactful alternative in terms of adjoining personal property.
- As the project topography slopes west to east, NBO encroaches on available space for SWM facilities on the downstream side of the project
- The eastern half of interchanges will require significant reconstruction with challenging vertical alignments and exacerbated bifurcation between steep exit/entrance ramps and northbound lanes

Concept NBO does reduce the amount of earthwork as compared to SBO, but at the expense of property impacts.

Again, the bridge improvement strategy drives the MOT/SOC approach. If replacement of bridges is preferred, it is expected the NBO concept will follow the same phasing approach as SBO. However, the reversible 'diversion' could be longer with NBO where I-81/bridges are raised to meet vertical clearance requirements and NB I-81 traffic will need to be put on a portion of the new SB I-81 structure temporarily, due to the bifurcation.

#### Preferred Strategy

RS&H recommends the SBO concept as the preferred alternative to progress to RFQ/RFP plan development.

The OO and NBO concepts are easily discounted due to high impacts east of I-81, including interchange ramps, residential and City of Salem properties, increased noise wall lengths, and hydraulic concerns. Concepts II and SBO were deemed feasible as compared to OO and NBO. Hence, a direct comparison was warranted for SBO vs. II is shown in Table 1 below. In an effort to examine attributes which benefit the corridor and traveling public well beyond initial capital costs, Table 1 lists important features beyond customary design or construction attributes. SBO is "better" than II for all but three features.

Category	Feature	SBO	II
Construction	Complexity of roadway sequence of construction	Better	Worse
Construction	Maintenance of Traffic	Better	Worse
Construction	Complexity of bridge sequence of construction	Better	Worse
Construction	Complexity of retaining wall type	Better	Worse
Construction	Erosion and Sediment Control complexity	Better	Worse
Construction	Storm Sewer System Complexity	Better	Worse
Construction	Sound wall expense	Similar	Similar
Construction	Earthwork balancing / reduction in borrow	Better	Worse
Construction	Order of Magnitude Construction cost	Similar	Similar
Construction	Design Exceptions	Better	Worse
Incident Mgmt.	Routing of vehicles on paved surface during accidents	Better	Worse
Incident Mgmt.	Access by tow / haul / accident cleanup vehicles	Better	Worse
Maintenance	Snow Removal - pushed snow melts across roadway	Better	Worse
Maintenance	Long term wall maintenance / Wall accessibility	Better	Worse

Table 1 –	Concept	Comparison	Chart



Category	Feature	SBO	Ш
Maintenance	Routine and long term storm sewer system	Better	Worse
Maintenance	Bridge maintenance and inspection	Better	Worse
Maintenance	MOT for maintenance work and general work area for routine maintenance	Better	Worse
Noise	Noise generation / noise reception	Similar	Similar
Right of Way	Number of parcels affected	Worse	Better
Safety	Total Shoulder Widths - all shoulders	Better	Worse
Safety	Paved Shoulder Widths - all shoulders	Better	Worse
Safety	Clear Distance to Infrastructure Obstructions	Better	Worse
Safety	Errant Truck into opposing lanes	Better	Worse
Safety	Recoverable area available for errant vehicles	Better	Worse
Safety	Truck breakdown areas available	Better	Worse
Safety	Speed enforcement locations for law enforcement	Better	Worse
Safety	Access by first responders during accidents	Better	Worse

Concept SBO marries the best features of the investigated concepts namely:

- Limits the impacts to interchanges and properties along the eastern portion of the corridor (City of Salem).
- Reduces the design complexity and construction of median barrier wall including associated drainage elements.
- More inclusive of safety components for this corridor given the **high truck volume (20-25%)** and existing 'highway safety corridor' designation.
  - o Produces improved crash modification factors (CMF) compared to II
  - Largely provides the standard inside and outside shoulders giving more recovery area for errant vehicles and potentially reducing 'run off road' and 'rear-end' type accidents
  - Limits length of walls with large vertical drops for errant trucks and cars in opposing directions
  - Allows first responders improved access to accidents and additional space to conduct investigations, cleanup efforts, and incident management traffic control
- Allows a more simplified/optimal construction sequence.
  - Improves operational elements by increasing available work area in the median so a larger amount of construction can occur in a single phase
  - Maintains the existing crown location for longer distances, reducing the need to shift the roadway crown throughout the corridor via pavement wedging. This reduces MOT steps/phases by eliminating the number of traffic shifts and lane closures needed during construction and eases impact to the traveling public.

- Affords phased bridge construction by constructing the NB bridges in two phases and the SB bridges potentially in a single phase without the need for 'over build', interchange ramp/loop closures, and large numbers of traffic switches.
- With regard to bridges, employing other concepts increases difficulty, as the bridges are and will continue to be independent structures due to bifurcation and horizontal clearance requirements, and current bridge/median width does not accommodate the construction of two new lanes in each direction (in the median). To maintain two travel lanes during construction, adequate new bridge width in a single direction needs to be provided within the median which requires at least one direction of I-81 to be widening to the outside at each bridge replacement location.
- Improves certain operational elements for the corridor including ultimate use, incident management scenarios, and maintenance activities.
  - Improves driver comfort by providing inside and outside shy line offset/distance for 65-70 MPH the majority of project length
  - More room to route traffic through incident areas via shoulders and travel lanes, reducing queue and delay times, potentially lessening the number of vehicles that divert to other routes and the impacts associated
  - Realization of greater flexibility and reduced impact to users during routine and major maintenance operations
- Increase in available room and accessibility for maintenance activities including snow removal, bridge inspection, bridge repair, median wall/enclosed drainage repair/cleaning, among others.
- Offers greater flexibility for the industry during procurement and final design. SBO's larger footprint (than II) gives Design Build offerors room to tailor a concept that may reduce impacts based on individual contractor means and methods. Providing a more constrained and complex RFP alternative to construct could result in cost and schedule risk as Design Builder innovation could be reduced.
- A more consistent typical section that generally conforms to VDOT/AASHTO cross sectional features specific to the inside shoulder widths. With only three localized exceptions, full width (12') inside shoulders are able to be provided through the corridor. Increased shoulder width was not a driving factor in the alternative selection, it is simply a realized benefit to the project and the corridor.



# Memorandum I-81 MM 136.6 to 141.8 D-B UPC 116203

To: Craig Moore (VDOT)
From: Tyler Gill
CC: Tony Dodson, Alex Vandyke (VDOT) Kyle Faby (RS&H)
Date: June 10, 2020
Re: SB I-81 Lane Shift (sta. 543+50 to 554+50)

Per request, this memo serves to document the design/construction related aspects of the SB I-81 12' lane shift between station 543+50 to 554+50.

#### **Design Background**

The RFP concept employs widening SB I-81 to the outside. As the project approaches the southern terminus in this direction, the need arises to terminate the added thru lane prior to the Texas Hollow Road bridge. Given, VDOT's preference is to 'drop' the third lane from the inside (median) the location of the baseline and/or lane configuration must be adjusted to tie to existing.

To accomplish this, the inside lane of SB I-81 is superelevated to create a planar roadway section north near sta. 554+50. Once planar, through large radius horizontal curves the location of the travel lanes are shifted 12' toward the median. After the shift, SB I-81 returns to a normal crown section with only a single lane west of the baseline.

The corresponding roadway plan sheet and typical sections are attached for reference.

#### **Design Features**

The following features/aspects are realized with the proposed lane shift.

- Lessens impacts to the Rt. 112 interchange, specifically the southbound loop ramp
- Reduces the need to further raise SB I-81 to achieve a 16'6" vertical clearance over Rt. 112
- Moves SB I-81 further away from Skyview Rd. and the associated cut slope thus reducing or eliminating the need for retaining structures or large right of way acquisition
- Allows the SB on ramp to utilize the existing auxiliary lane infrastructure
- Reduces/eliminates large impacts to the acid producing soils that are present in the cut slope between Exit 137 and the Texas Hollow bridges
- Provides flexibility in location of where the thru lane is truncated
- Maintains a crowned roadway section, easing constructability at all points south of the shift

- Eases spread concerns at the Rt. 112 bridge by returning to a normal crown cross section prior to the structure
- Accommodates an inside/inside widening concept moving south
- Results in a reduced inside shoulder for approximately 1500', requiring a design waiver/design exception

#### Attachments

- RFQ Plan Sheet 5
- RFQ Typical Section Sheets 2A(1-2)



# I-81 Corridor-wide Design Guide

Appendix C





# IMPROVE 81

# Appendix C: DE/DW for Bridge Clearance

#### Information needs:

- Most recent Vertical Clearance (VC) figures from the last bridge inspection report
- Existing roadway plans showing typical section and vertical curve/grade information at the structure
- Survey data of existing low chord of bridge and corresponding ground elevation of under-route
- Crash history of any over height bridge impacts
- VC of adjacent structures up and down stream of project
- Traffic volumes, including any truck percentages and adjacent sites requiring truck access
- Comprehensive Plan (Future land-use Map)

#### **References:**

- AASHTO Green Book
- VDOT S&B Manual Part 1
- VDOT S&B Manual Part 2, Ch 6
- VDOT IIM-LD-227/IIM-S&B-70

#### Process:

- Determine existing route classification of both over and under routes
- Verify existing VC
- Interpolate proposed VC given
  - Width of widening
  - Proposed cross slope in direction of widening
  - o Existing beam/deck depth
  - o Proposed under-route geometrics
- Review new VC against S&B requirements
- If VC greater than minimum for route, process complete
- If VC less than minimum for route per S&B guidance, consider:
  - o Can superstructure (beams) be made shallower to accommodate minimum?
    - If yes Incorporate into design, no DE/DW required
  - Can superstructure (beams) be made shallower to meet or exceed existing VC for structure or twin structures?
    - If yes Consider crash history of any over height impacts and produce DE/DW
      - Further consideration may be needed if over height impacts are significant
    - If no DE/DW will be required and should consider:

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# IMPROVE 81

- Crash history of any over height impacts
- VC up and down stream of structure(s)
- Any future projects/studies for either route
- Conceptual cost of lowering under route
- Conceptual costs of superstructure jacking or bearing replacement including cost of associated roadway leveling
- Conceptual cost of a superstructure/bridge replacement as a last resort

#### **DE/DW Formatting:**

- All DE/DWs should be addressed to State S&B Engineer.
- Verify proper forms (LD-440 or LD-448) based upon Chapter 6 of S&B manual for corresponding clearance and roadway classification.
- Cost data provided can be rough order of magnitude (ROM) level for justification of DE/DW.
- ROM costs should include potential for additional RW, utility and environmental impacts.
- Writeup sections (background, design criteria, justifications, etc.) should be provided as an attachment to the appropriate form.
- Roadway plans, bridge layouts or VC sketches, traffic projections should be included as part of an appendix.

#### Notes:

- Initial determination can be made without survey data, but final design should incorporate.
- If final design yields different VC than DE/DW, team should discuss with S&B for need to modify DE/DW.
- All VC DE/DWs require State S&B Engineer approval regardless of roadway classification.

A sample approved DE/DW is attached at the end of this section.

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#### VIRGINIA DEPARTMENT OF TRANSPORTATION LOCATION AND DESIGN/STRUCTURE & BRIDGE DESIGN WAIVER REQUEST (See IIM-LD-227 for the definition of Design Waiver) Design Waiver Number: (For Use by NOVA Only)

Date: 9/8/2020						
To:	Kendal Walus	5, PE			State Structure	and Bridge Engineer
From Project	Designer (L&D	, S&B or Consultant):	Danie	el G Da	avis, PE - PRIME A	AE Group
Project Info	rmation					
UPC 116203 State Project 0081-080-946, Number		0081-080-946, B684, B685				
Federal Proje	ct Number	NHPP-0812(323) District Sale		Salem		
City/County		Roanoke County/City of Salem				
Project Descr	iption	I-81 Widening MM 136.6 to 141.8				
Start Location	n (From)	0.237 Mi. North Rt. 641				
End Location	(То)	0.349 Mi. North Rt. 419				
Funding Sour	се	Federal				

Road Information						
Func	Functional Classification         GS-7 URBAN COLLECTOR STREET		Minin	num VDOT GS St'd	GS-7	
Min.	VDOT Standard	16'-6"		VDOT	Reference Location	BDM 06.02-010
Desi	gn Speed	35 mpł 30 mpł	(Roanoke County) / (City of Salem)	Posted Speed		35 mph (Roanoke County) 25 mph (City of Salem)
Desi	gn Waiver Request	For The	Following			
	Minimum Radius		Lane Shift/Tapers		Total Shoulder Wig	dth
	Buffer Strip Width		Ditch Width		Bike & Ped Accommodations Compliance IIM-LD-55 & RDM )	
	Paved Shoulder Wic	ith	Superelevation	Guardrail GR-9/GR-2		
	Curb and Gutter		Intersection Sight Distance			
$\boxtimes$	Other	er Vertical Clearance				

Design Waiver request must address the following:

- Established design criteria versus proposed and existing criteria (including traffic data, design speed and posted speed)
- Reason the appropriate design criteria cannot be met
- Justification for the proposed criteria
- Any background information which documents, supports or justifies the request
- Any mitigation that will be provided to further support or justify the request
- Cost to meet standard versus project cost

Attach all supporting documentation to this exhibit including crash history (past three years). Design waivers for the use of GR-9 terminals or GR-2 guardrails as described in the Memorandum dated January 3, 2018 <u>Clarification of MASH guardrail requirements</u>, do not require the following information listed above for the design waver: GS standard, traffic data, crash history, design speed, or posted speed.



Prepared By:

Daniel G Davis, PE Click to choose an item. Date: 9/10/2020

Note: The responsible person that prepares the request shall also electronically seal and digitally sign in the block above. All signatures below shall be digital signatures.

LD-448 (5-22-20)

VDOT Approved By: _____

Click to choose an item.

CC: Appropriate Assistant State Location and Design Engineer Project Manager State Geometric Design Engineer State Structure and Bridge Engineer Assistant State Traffic Engineer – Traffic Control Devices

# Background/Project Description

UPC 116203 proposes to widen NB and SB I-81 by a single lane in each direction between mile marker 136.6 and 141.8, approximately 5.2 miles. This project begins about 3,000 feet south of the I-81/Rt. 112 (Wildwood Rd.) interchange (Exit 137) near the I-81 over Rt. 641 (Texas Hollow Rd.) bridges and ends about 1,800 feet north of the I-81/Rt. 419 (North Electric Rd.) interchange. The project is located within Roanoke County and the City of Salem, as the corporate limits straddle I-81 through this portion of the corridor.



Three interchanges are found within the project: (1) Exit 137 – I-81/Rt. 112 – Partial Cloverleaf/Folded Diamond, (2) Exit 140 – I-81/Rt. 311 – Partial Cloverleaf/Folded Diamond, and (3) Exit 141 – I-81/Rt. 419 – Partial Cloverleaf. In addition to the interchanges, there are three underpasses (Rt. 635 – Goodwin Ave., Rt. 619 – Wildwood Rd./Academy St., Rt. 630 – Kessler Mill Rd.) and one overpass (Rt. 705 – Red Ln.). Included in UPC 116203 is the replacement of the bridge structures at Rt. 112 (Exit 137), Rt. 635, and Rt. 619. The Rt. 311 (Exit 140) bridges will be widened to accommodate the additional thru lane. No proposed widening or replacement work will occur on the Rt. 705, Rt. 630, or Rt. 419 (Exit 141) bridges.

The proposed widening scheme generally follows constructing an additional lane to the outside in the SB direction and to the inside in the NB direction. The typical section is mainly comprised of three 12' lanes and two 12' total shoulders per direction.

UPC 116203 was identified as Project ID #39A and 39B in the 2018 'I-81 Corridor Improvement Plan', with the goal to provide additional capacity, reduce congestion, and improve safety.
Immediately to the north of this project another, UPC 108906, is currently under construction to widen and connects the auxiliary lanes together between Exits 141 and 143. The proposed design for UPC 116203 will tie into the UPC 108906 at the northern terminus.

This design waiver seeks a reduction in the required vertical clearance from 16'-6" to 15'-8" for the NB and SB I-81 bridges over Route 635 (Goodwin Ave.), as presented in the RDM GS-7 and BDM 06.02-09-10.

#### Established Design Criteria

Route 635 (Goodwin Ave) between Route 11/460 (West Main Street) and Route 619 (Wildwood Road) is classified as an Urban Collector Street (GS-7) with a design/posted speed of 35 MPH within Roanoke County and a design speed of 30 MPH (posted of 25 MPH) within the City of Salem.

The 2043 forecasted traffic volumes for Route 635 is 990 ADT while the design year traffic volumes is 875 vehicle per day with 2% trucks (See Table 1).

Route 635											
From:	NCL Salem										
To:	Route 619										
ADT (2020)	875										
ADT (2045)	990										
DHV	115										
D (% Design Hour)	79										
T (% Design Hour)	2%										

Table	<b>1</b> :	Route	635	Traffic	Data
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Interstate 81 between MM 136.6 and 141.8 is classified as Urban Interstate System (GS-INT) with a design speed of 65 MPH and a posted of 60 MPH.

The 2043 forecasted traffic volumes for NB I-81 is 31,882 ADT while the design year traffic volumes for SB I-81 is 27,038 ADT. See Appendix D for the project LD-104 report.

According to BDM 06.02-10, urban collector streets (GS-7) shall have a minimum vertical clearance of 14'-6" and the desirable minimum vertical clearance of 16'-6". Vertical clearances between these values require a design waiver.

#### Proposed Design Criteria

Provide a minimum 15'-8" vertical clearance for the NB and SB I-81 bridges over Route 635, meeting the existing vertical clearance of the NB I-81 over Route 635 bridge.

#### Reason The Established Design Criteria Cannot Be Met

The existing I-81 and Route 635 vertical alignments do not accommodate a proposed 16'-6" vertical clearance without significant impacts to the grades of I-81 and Route 635. Both the NB and SB I-81 bridges over Rt. 635 have existing sub-standard vertical clearances denoted in the figures below from the inspection reports. As depicted, the existing vertical clearance for the NB I-81 bridge over Route 635 is 15'-8" while the SB I-81 bridge over Route 635 is 13'-7".

Adjusting the I-81 vertical alignments to provide a 16'-6" vertical clearance requires raising the existing SB I-81 grade approximately 3' and the NB I-81 grade 10" which results in larger asphalt build-up depths, increased full depth pavement replacement, wider construction limits, additional right of way/easement needs, taller bridge structures, and affects the location of SWM BMPs. Given two lanes of I-81 must be maintained in each direction during construction, these features coupled with the existing bifurcation and narrow median increase overall MOT/SOC complexity, duration, and impact to the traveling public throughout this phase of the project. In addition, as this project adds another thru lane in each direction to I-81, the proposed vertical alignments need to be raised further to accommodate the cross slope of a third lane.

Attempting to increase the vertical clearance at these crossings by lowering the existing Route 635 vertical alignment is met with its own challenges. Among them are steep existing vertical grades that complicate profile adjustment, additional right of way/easement needs in a high residential area east of I-81 and steep cut slope west of I-81, potential right of way impacts to 'paper streets' depicts on subdivision plats west of I-81, and culvert replacement and environmental/stream impacts east of I-81.

#### CLEARANCE SHEET



LEGEND: EP = EDGE OF PAVEMENT * = EDGE OF PAVED SHOULDER FC = FACE OF CURB

ROUTE	I-81 NBL
DVER:	RTE 635
	ROANOKE
STRUCT, ND. 1	2010
INITIALS	JEP
CADD DWG. :	80-2010 VC

Figure 1: Sketch from 2019 Bridge Inspection Report Route I-81 NBL over Route 635

#### CLEARANCE SHEET



LEGEND: EP = EDGE OF PAVEMENT * = EDGE OF PAVED SHOULDER FC = FACE OF CURB

ROUTE	I-81 SBL
OVER:	RTE 635
	ROANOKE
STRUCT, ND. 1	2011
INITIALS	JEP
CADD DWG. :	80-2011 VC

Figure 2: Sketch from 2019 Bridge Inspection Report Route I-81 SBL over Route 635

#### Justification For The Proposed Design Criteria

To reduce overall construction impacts to I-81 and Route 635, the proposed design employs a hybrid approach through the lowering of the Route 635 and raising of the I-81 profiles to achieve a 15'-8" vertical clearance at both bridges. This provides an optimal solution for reducing bridge, right of way, environmental, and constructability impacts.

With this approach, the NB and SB I-81 vertical alignments are able to mimic the existing grades more closely through this crossing, drastically reducing the required amount of full depth pavement and utilizing a more constructable mill and overlay application. The proposed I-81 profiles allow for improved side slope ties thus reducing right of way/easement needs associated with grade changes in this area. On Route 635, the proposed profile gains valuable vertical clearance under the bridges while not impacting the culvert/stream east of I-81 and the steep cut slope west of I-81.

Additionally, a review of the Route 635 existing ADT shows low volumes that are not expected to exceed 1000 vehicles per day by the design year and a truck volume of only 2%. Further study of this area identifies Route 619 (Wildwood Rd./Academy St.) as a viable alternative route for vehicle with heights exceeding 15'-8". Route 619 is within 0.5 miles of the Route 635/I-81 crossing and provides an existing (and proposed) vertical clearance of more than 20'.

Further, the 2019 I-81 over Route 635 Bridge Inspection Reports note evidence of a vehicle strike to the SB I-81 structure (13'-7" vertical clearance) but not to the NB I-81 bridge (15'-8" vertical clearance). It should be noted, however, that there are no records of vehicle strikes to either of these existing bridges.

The proposed design incorporates a context sensitive approach to these bridge structures by balancing constructability, route usage, and proposed vertical clearance to deliver a design that improves the existing condition while exceeding the AASHTO minimum of 14'-6".

#### **Background Information**

Included as appendices to this design waiver are:

- Appendix A: RFP Plan and Profile Sheets for I-81 Crossing Route 635
- Appendix B: Satellite view of Route 619 as alternate truck route
- Appendix C: Rt. 635 Goodwin Ave Vertical Clearance Alternative Discussion Memo
- Appendix D: LD-104 I-81 Traffic Data for Scoping Report and Design Purposes

### Mitigation That Will Be Provided To Further Support Or Justify The Request

FHWA's Technical Report on Mitigation Strategies for Design Exceptions identifies the use of advance warning signs for vertical clearance.

The proposed design will include advance warning signage to alert drivers of the 15'-8" vertical clearance approaching the I-81/Route 635 crossing in both directions.

#### Cost To Meet Standard Versus Project Cost

Due to available funding being constrained, project cost increases have severe consequences to overall project viability. Given current project cost estimate are very near the allocated SYIP funds, further cost increases jeopardize the project. The current construction estimate for this project is \$300 million.

The additional cost to meet a 16'-6" vertical clearance would require approximately an additional \$1-2 million in associated roadway and bridge construction costs as compared to the 15'-8" option, resulting in an increase of approximately 1% in total construction cost. However, other cost impacts such as design fees, right of way, environmental, and owner costs are not included in this estimate.







REFEREN (PROFILES,DETAIL DESCRIPTION SH	ICES L & DRAINAGE IEETS, ETC.)
Utility Owners	IF(2)
Construction Alignment Data	IG(1) - IG(9)
I-8I Profile	7A
Rt.635 Profile	IIB
Structural Obstruction Zone	7 <i>S0Z</i>









SHEET NO.



Roanoke County / City of Salem



Project Location

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Scale



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Legend

Proposed Bridge

RFP PLANS For Information Only DATE: 09/17/20

THESE PLANS ARE UNFINISHED AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY.

ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT- OF-WAY SHOWN ON THESE PLANS.

Contact Information

Craig Moore. PE Project Manager / Assistant District L&D Engineer Virginia Department of Transportation 731 Harrison Avenue Salem, VA 24153

Comments may also be sent to: I81-MM136-141@vdot.virginia.gov

I-81 Widening MM 136.6 to 141.8 Roanoke County / City of Salem, Virginia 0081-080-946, P101, R201, C501 UPC 116203 Federal Project Number

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# PROFILE ROLL I-81 WIDENING MM 136.6 TO 141.8



# Appendix A

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Roanoke County / City of Salem

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RFP PLANS For Information Only DATE: 09/17/20 THESE PLANS ARE UNFINISHED

AND UNAPPROVED AND ARE NOT TO BE USED FOR ANY TYPE OF CONSTRUCTION OR THE ACQUISITION OF RIGHT OF WAY.

ADDITIONAL EASEMENTS FOR UTILITY RELOCATIONS MAY BE REQUIRED BEYOND THE PROPOSED RIGHT- OF-WAY SHOWN ON THESE PLANS.

**Contact Information** 

Craig Moore. PE Project Manager / Assistant District L&D Engineer Virginia Department of Transportation 731 Harrison Avenue Salem, VA 24153

Comments may also be sent to: l81-MM136-141@vdot.virginia.gov

I-81 Widening MM 136.6 to 141.8 Roanoke County / City of Salem, Virginia 0081-080-946, P101, R201, C501 UPC 116203 Federal Project Number

# **PROFILE ROLL** I-81 WIDENING MM 136.6 TO 141.8

# Appendix A

## **RT. 311 RAMP A**





**RT. 311 RAMP B** 

# RT. 635 (GOODWIN AVE.)

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### **RT. 311 LOOP D**



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IB

#### Appendix B Alternate Truck Route





#### Memorandum I-81 MM 136.6 to 141.8 D-B UPC 116203

To:	File
From:	Tyler Gill
CC:	VDOT Project Team
	RS&H Design Team
Date:	July 2, 2020
Re:	Meeting Notes/Design Direction: Rt. 635 Goodwin Ave Vertical Clearance Alternative Discussion

This memo documents meeting notes and design direction that was agreed to at the 'Rt. 635 Goodwin Ave Vertical Clearance Alternative Discussion' on 2020-07-01. The meeting was held virtually via MS Teams and included VDOT Salem District, VDOT CO, VDOT APD, and RS&H Design Team staff. An attendee list is included as Appendix A.

#### **Meeting Minutes/Notes**

The goal of this meeting was to determine the vertical clearance to be accommodated at the I-81 over Rt. 635 crossing for inclusion into the RFP plans and technical requirements.

Craig Moore provided a brief background and introduction of the alternatives:

- 16' 6" Meets VDOT minimum vertical clearance
- 15' 8" Holds vertical clearance currently provided by the NB I-81 structure (requires design waiver)
- 14' 6" Meets AASHTO minimum vertical clearance (requires design waiver)

Mr. Moore communicated that the localities were not supportive of an alternative that closed Goodwin on each side of I-81.

RS&H (Tyler Gill) presented each alternative in greater detail, discussing the Vertical Clearance Analysis (attached for reference in Appendix B) that was completed and the effect each alternative had on the NB I-81, SB I-81, and Rt. 635 vertical alignments as well as the corresponding cost estimate impacts.

The group had discussion that included constructability, construction sequencing, technical requirement language, DBT incentives, infrastructure protection, and DBT approach to optimizing bridge structures and vertical clearances.

VDOT S&B noted that only the SB I-81 structure had evidence of bridge strikes and thus was supportive of a 15' 8" clearance.

#### **Vertical Clearance Decision**

All parties agreed to providing **a minimum 15' 8'' vertical clearance** for the I-81 structures crossing Rt. 635 (B684 & B685)

The design team will incorporate the 15' 8" vertical clearance into the development of the RFP plans. Technical requirement language will specify the required minimum clearance at this location.



#### COMMONWEALTH of VIRGINIA

DEPARTMENT OF TRANSPORTATION 731 HARRISON AVENUE SALEM, VIRGINIA 24153

Stephen Brich, P.E. COMMISSIONER

June 15, 2020

To: Craig Moore Design Engineer

From: Carol J.L. Moneymaker Planning Specialist Carol J.L. Moneymaker

Subject:	Traffic Data for Scoping Report and Design Purposes
Route:	I-81
Project:	0081-080-946
UPC:	116203
Location:	Roanoke County
From:	Overpass of Route 641 (MP 136.85)
To:	0.3 MI north of Route 419 Overpass (MP 141.91)

Listed below is the requested data for the project referenced above. For mainline counts, text in *italics* indicates data derived from counts taken in 2017; all other mainline data is calculated from counts taken in 2019. All ramp data is calculated from counts taken in 2017.

I-81 Between Exits 132 and 137	Northbound	Southbound	Combined
Current ADT (2019)	27,296	23,149	50,445
Design Year ADT (2043):	31,882	27,038	58,920
Design Hourly Volume (DHV):	N/A	N/A	5,503
Directional DHV (Trucks):	N/A	N/A	901
Existing AM Peak Hour Traffic:	1,745	1,603	3,342
Existing AM Peak Hour Period	7:15 - 8:15	7:30 - 8:30	7:15 - 8:15
Existing PM Peak Hour Traffic:	2,235	2,303	4,507
Existing PM Peak Hour Period:	4:15 - 5:15	4:30 - 5:30	4:30 - 5:30
AM Directional Distribution Factor*	1,745 (52%)	1,597 (48%)	3,342
PM Directional Distribution Factor*	2,204 (49%)	2,303 (51%)	4,507
Peak Hour Factor:	0.98	0.93	0.97

Truck Percentage (Northbound)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		4.0% 4.1%	18.3% 25.8%	22.3% 29.9%
Truck Percentage (Southbound)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		2.5% 4.1%	14.3% 22.4%	16.8% 26.5%
Truck Percentage (Combined)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		3.0% 4.1%	16.4% 24.1%	19.4% 28.2%
I-81 Between Exits 137 and 140	Northbound	Sout	hbound	Combined
Current ADT (2019)	31,659	31,7	65	63,424
Design Year ADT (2043):	Design Year ADT (2043): 41,537 41,676		76	83,212
Design Hourly Volume (DHV):	N/A	N/A		6,823
Directional DHV (Trucks):	N/A	N/A		922
Existing AM Peak Hour Traffic:	2,100	1,98	1	4,081
Existing AM Peak Hour Period	7:30 - 8:30	7:30	- 8:30	7:30 - 8:30
Existing PM Peak Hour Traffic:	2,596	2,86	9	5,458
Existing PM Peak Hour Period:	4:15 - 5:15	4:45	- 5:45	4:45 - 5:45
AM Directional Distribution Factor*	2,100 (51%)	1,98	1 (49%)	4,081
PM Directional Distribution Factor*	2,589 (47%)	2,86	9 (53%)	5,458
Peak Hour Factor:	0.98	0.94		0.97
Truck Percentage (Northbound)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		3.2% 4.0%	16.8% 23.2%	19.9% 27.2%
Truck Percentage (Southbound)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.0% 1.7%	0.7% 1.2%	12.8% 20.0%	14.4% 22.9%
Truck Percentage (Combined)		<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily		2.2% 3.5%	14.6% 21.5%	16.8% 25.0%

I-81 Between Exits 140 and 141	Northbound	Sout	hbound	Combined
Current ADT (2019)	33,436	33,5	87	67,023
Design Year ADT (2043):	43,066	43,2	60	86,326
Design Hourly Volume (DHV):	N/A	N/A		7,856
Directional DHV (Trucks):	N/A	N/A		1,064
Existing AM Peak Hour Traffic:	2,626	2,36	1	5,005
Existing AM Peak Hour Period	7:30 - 8:30	7:15	- 8:15	7:15 - 8:15
Existing PM Peak Hour Traffic:	2,703	2,76	2	5,436
Existing PM Peak Hour Period:	4:00 - 5:00	4:30	- 5:30	4:15 - 5:15
AM Directional Distribution Factor*	2,644 (53%)	2,36	1 (47%)	5,005
PM Directional Distribution Factor*	2,679 (49%)	2,75	7 (51%)	5,436
Peak Hour Factor:	0.97	0.96		0.97
Truck Percentage (Northbound)	Class 4-5	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	2.1% 2.0%	0.8% 1.2%	18.1% 24.1%	21.0% 27.3%
Truck Percentage (Southbound)	<u>Class 4-5</u>	Class 6-7	<u>Class 8-13</u>	Total
% Peak % Daily	1.4% 2.1%	1.4% 1.5%	14.4% 22.1%	17.3% 25.7%
Truck Percentage (Combined)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.8% 2.0%	1.2% 1.3%	16.3% 23.1%	19.2% 26.5%

I-81 Between Exits 141 and 143	Northbound	Sout	hbound	Combined
Current ADT (2019)	36,787	36,3	68	73,155
Design Year ADT (2043):	47,382	46,8	42	94,224
Design Hourly Volume (DHV):	N/A	N/A		8,603
Directional DHV (Trucks):	N/A	N/A		1,056
Existing AM Peak Hour Traffic:	3,088	2,78	6	5,874
Existing AM Peak Hour Period	7:15 - 8:15	7:15	- 8:15	7:15 - 8:15
Existing PM Peak Hour Traffic:	3,068	3,07	4	6,142
Existing PM Peak Hour Period:	4:15 - 5:15	4:15	- 5:15	4:15 - 5:15
AM Directional Distribution Factor*	3,088 (53%)	2,78	6 (47%)	5,874
PM Directional Distribution Factor*	3,068 (50%)	3,07	4 (50%)	6,142
Peak Hour Factor:	0.96	0.94		0.97
Truck Percentage (Northbound)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	Total
% Peak % Daily	1.5% 1.8%	0.7% 1.2%	14.7% 21.9%	16.9% 24.9%
Truck Percentage (Southbound)	<u>Class 4-5</u>	Class 6-7	<u>Class 8-13</u>	Total
% Peak % Daily	1.6% 2.0%	0.8% 1.2%	14.2% 20.4%	16.7% 23.6%
Truck Percentage (Combined)	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.6% 1.9%	0.8% 1.2%	14.5% 21.1%	16.8% 24.2%

#### Ramp 1 - Exit 137: I-81 NB Exit Ramp to Route 112 (Wildwood Road)

Current ADT (2017):	2,590		
Design Year ADT (2043):	3,061		
Design Hourly Volume (DHV):	301		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	<u>Total</u>
% Peak % Daily	0.0% 3.6%	0.0% 4.9%	0.0% 8.5%
Existing Peak Hour Traffic:	A.M.: 171 P.M.: 213	(7:00 AM - 8 (4:30 PM - 5:	:00 AM) 30 PM)
Peak Hour Factor:	0.92		

#### Ramp 2 - Exit 137: I-81 NB Entrance Ramp from Route 112 (Wildwood Road)

Current ADT (2017):	7,232		
Design Year ADT (2043):	8,548		
Design Hourly Volume (DHV):	676		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.6% 4.3%	0.9% 5.9%	1.5% 10.2%
Existing Peak Hour Traffic:	A.M.: 587 P.M.: 645	(7:15 AM - 8: (4:30 PM - 5:	15 AM) 30 PM)
Peak Hour Factor:	0.92		

#### Ramp 3 - Exit 137: I-81 SB Exit Ramp to Route 112 (Wildwood Road)

Current ADT (2017):	6,729				
Design Year ADT (2043):	7,954	7,954			
Design Hourly Volume (DHV):	764				
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	<u>Total</u>		
% Peak % Daily	0.7% 4.7%	1.0% 6.0%	1.7% 10.8%		
Existing Peak Hour Traffic:	A.M.: 492 P.M.: 679	(7:30 AM - 8 (4:45 PM - 5	3:30 AM) :45 PM)		
Peak Hour Factor:	0.92				

#### Ramp 4 - Exit 137: I-81 SB Entrance Ramp from Route 112 (Wildwood Road)

Current ADT (2017):	2,349			
Design Year ADT (2043):	2,777	2,777		
Design Hourly Volume (DHV):	243			
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	<u>Total</u>	
% Peak % Daily	0.3% 4.1%	0.8% 4.2%	1.0% 8.2%	
Existing Peak Hour Traffic:	A.M.: 189 P.M.: 191	(7:15 AM - 8 (4:45 PM - 5	8:15 AM) 5:45 PM)	
Peak Hour Factor:	0.90			

#### Ramp 5 - Exit 140: I-81 NB Exit Ramp to Route 311 (Thompson Memorial Drive)

Current ADT (2017):	2,608		
Design Year ADT (2043):	3,489		
Design Hourly Volume (DHV):	435		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.0% 4.4%	0.0% 2.0%	1.0% 6.4%
Existing Peak Hour Traffic:	A.M.: 245 P.M.: 243	(7:00 AM - 8: (5:00 PM - 6:0	00 AM) 00 PM)
Peak Hour Factor:	0.89		

#### Ramp 6 - I-81 NB Entrance Ramp from Route 311 (Thompson Memorial Drive)

Current ADT (2017):	4,303			
Design Year ADT (2043):	5,757	5,757		
Design Hourly Volume (DHV):	646			
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>	
% Peak % Daily	0.3% 2.6%	0.1% 1.0%	0.4% 3.6%	
Existing Peak Hour Traffic:	A.M.: 410 P.M.: 480	(7:30 AM - 8 (5:30 PM - 6	3:30 AM) :30 PM)	
Peak Hour Factor:	0.85			

Ramp 7 - I-81 SB Exit Ram	to Route 311 (Th	hompson Memorial Drive)

Current ADT (2017):	4,291		
Design Year ADT (2043):	5,741		
Design Hourly Volume (DHV):	640		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.1% 2.3%	0.0% 1.0%	0.1% 3.3%
Existing Peak Hour Traffic:	A.M.: 389 P.M.: 442	(7:30 AM - 8 (5:45 PM - 6	:30 AM) :45 PM)
Peak Hour Factor:	0.80		

#### Ramp 8 - I-81 SB Entrance Ramp from Route 311 (Thompson Memorial Drive)

Current ADT (2017):	2,412		
Design Year ADT (2043):	3,227		
Design Hourly Volume (DHV):	344		
Truck Percentage:	Class 4-7	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.4% 3.9%	0.7% 2.8%	1.1% 6.6%
Existing Peak Hour Traffic:	A.M.: 198 P.M.: 271	(7:15 AM - 8: (4:30 PM - 5::	15 AM) 30 PM)
Peak Hour Factor:	0.90		

#### Ramp 9 - I-81 NB Exit Ramp to Route 419 (Electric Road)

Current ADT (2017):	2,725		
Design Year ADT (2043):	3,645		
Design Hourly Volume (DHV):	370		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.5% 5.3%	0.0% 4.8%	1.5% 10.0%
Existing Peak Hour Traffic:	A.M.: 231 P.M.: 270	(7:30 AM - 8: (4:30 PM - 5:	30 AM) 30 PM)
Peak Hour Factor:	0.86		

#### Ramp 10 - I-81 NB Entrance Ramp from Route 419 (Electric Road)

Current ADT (2017):	6,214		
Design Year ADT (2043):	8,314		
Design Hourly Volume (DHV):	826		
Truck Percentage:	<u>Class 4-7</u>	Class 8-13	Total
% Peak % Daily	0.3% 3.1%	0.2% 1.9%	0.6% 5.0%
Existing Peak Hour Traffic:	A.M.: 582 P.M.: 612	(7:15 AM - 8 (4:30 PM - 5	8:15 AM) :30 PM)
Peak Hour Factor:	0.85		

#### Ramp 11 - I-81 SB Exit Ramp to Route 419 (Electric Road)

Current ADT (2017):	6,262		
Design Year ADT (2043):	8,380		
Design Hourly Volume (DHV):	959		
Truck Percentage:	<u>Class 4-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.6% 3.2%	0.4% 1.8%	1.0% 5.0%
Existing Peak Hour Traffic:	A.M.: 629 P.M.: 590	(7:15 AM - 8: (4:30 PM - 5:3	15 AM) 30 PM)
Peak Hour Factor:	0.85		

#### Ramp 12 - I-81 SB Entrance Ramp from Route 419 (Electric Road)

Current ADT (2017):	2,902		
Design Year ADT (2043):	3,882		
Design Hourly Volume (DHV):	355		
Truck Percentage:	Class 4-7	Class 8-13	Total
% Peak % Daily	0.4% 4.6%	0.7% 4.6%	1.1% 9.2%
Existing Peak Hour Traffic:	A.M.: 239 P.M.: 277	(7:15 AM - 8: (4:45 PM - 5:	15 AM) 45 PM)
Peak Hour Factor:	0.94		

Route 112 (Wildwood Road) from NCL Sal	em to I-81			
Current ADT (2019):	20,360			
Design Year ADT (2043):	24,406			
Design Hourly Volume (DHV):	2,060			
Directional DHV (Trucks):	17			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	Cannot be cale 1.2%	culated with ave 0.1%	ailable count da 0.2%	ata. 1.5%
Directional Distribution Factor:	A.M.: 1,804 P.M.: 1,801	N: 870 (48%) N: 828 (46%)	S: 934 (52%) S: 973 (54%)	
Existing Peak Hour Traffic:	A.M.: 1,804 P.M.: 1,801	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
Peak Hour Factor:	0.90			

#### Route 112 (Wildwood Road) from I-81 to FR 70

Current ADT (2019):	908			
Design Year ADT (2043):	1,017			
Design Hourly Volume (DHV):	127			
Directional DHV (Trucks):	2			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	1.5% 1.4%	0.5% 0.6%	0.0% 0.1%	2.0% 2.1%
Directional Distribution Factor:	A.M.: 82 P.M.: 101	N: 18 (22%) N: 63 (62%)	S: 64 (78%) S: 38 (38%)	
Existing Peak Hour Traffic:	A.M.: 82 P.M.: 101	(7:15 AM - 8: (5:15 PM - 6:	15 AM) 15 PM)	
Peak Hour Factor:	0.90			

				(10) (W(11) 1D 1)
UK8051 (Acad	emy Street) Ir	om w. Carrolton	Avenue to Route (	519 (Wildwood Koad)

Current ADT (2019):	1,950			
Design Year ADT (2043):	2,225			
Design Hourly Volume (DHV):	240			
Directional DHV (Trucks):	2			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.5% 1.2%	0.0% 0.1%	0.0% 0.1%	0.5% 1.4%
Directional Distribution Factor:	A.M.: 218 P.M.: 186	N: 70 (32%) N: 113 (61%)	S: 148 (68%) S: 73 (39%)	
Existing Peak Hour Traffic:	A.M.: 218 P.M.: 186	(7:15 AM - 8: (5:00 PM - 6:0	15 AM) 00 PM)	
Peak Hour Factor:	0.88			

#### Route 619 (Wildwood Road) from Route 733 to UR 8051 (Academy Street)

Current ADT (2019):	1,190			
Design Year ADT (2043):	1,333			
Design Hourly Volume (DHV):	154			
Directional DHV (Trucks):	1			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.4% 0.9%	0.4% 0.4%	0.7% 0.2%	1.4% 1.5%
Directional Distribution Factor:	A.M.: 112 P.M.: 140	E: 82 (73%) E: 63 (45%)	W: 30 (27%) W: 77 (55%)	
Existing Peak Hour Traffic:	A.M.: 112 P.M.: 140	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
Peak Hour Factor:	0.78			

Route 311 from Rose Ln to NCL Salem				
Current ADT (2019):	13,707			
Design Year ADT (2043):	15,352			
Design Hourly Volume (DHV):	1,640			
Directional DHV (Trucks):	17			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak	0.7%	0.1%	0.5%	1.2%
% Daily	1.0%	0.3%	0.7%	2.0%
Directional Distribution Factor:	A.M.: 1,462 P.M.: 1,580	N: 604 (41%) N: 875 (55%)	S: 858 (59%) S: 705 (45%)	
Existing Peak Hour Traffic:	A.M.: 1,462 P.M.: 1,580	(7:15 AM - 8: (4:30 PM - 5:3	15 AM) 30 PM)	
Peak Hour Factor:	0.94			

#### Route 311 from NCL Salem to Route 419

Current ADT (2019):	5,934			
Design Year ADT (2043):	6,646			
Design Hourly Volume (DHV):	750			
Directional DHV (Trucks):	8			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak	0.7%	0.1%	0.5%	1.2%
% Daily	1.0%	0.3%	0.7%	2.0%
Directional Distribution Factor:	A.M.: 486 P.M.: 603	N: 162 (33%) N: 349 (58%)	S: 324 (67%) S: 254 (42%)	
Existing Peak Hour Traffic:	A.M.: 486 P.M.: 603	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 45 PM)	
Peak Hour Factor:	0.94			

#### Route 419 from NCL Salem to I-81

	Current ADT (2019):	12,773			
	Design Year ADT (2043):	14,306			
	Design Hourly Volume (DHV):	1,702			
	Directional DHV (Trucks):	45			
	Truck Percentage:	<u>Class 4-5</u>	Class 6-7	<u>Class 8-13</u>	Total
	% Peak % Daily	0.4% 1.4%	0.3% 0.8%	1.3% 2.6%	2.0% 4.8%
	Directional Distribution Factor:	A.M.: 1,428 P.M.: 1,511	N: 640 (45%) N: 827 (55%)	S: 788 (55%) S: 684 (45%)	
	Existing Peak Hour Traffic:	A.M.: 1,428 P.M.: 1,511	(7:15 AM - 8: (4:45 PM - 5:4	15 AM) 15 PM)	
	Peak Hour Factor:	0.95			
Route 4	419 from I-81 to Route 311				

Current ADT (2019):	9,353			
Design Year ADT (2043):	10,475			
Design Hourly Volume (DHV):	983			
Directional DHV (Trucks):	30			
Truck Percentage:	<u>Class 4-5</u>	<u>Class 6-7</u>	<u>Class 8-13</u>	<u>Total</u>
% Peak % Daily	0.4% 1.4%	0.3% 0.8%	1.3% 2.6%	2.0% 4.8%
Directional Distribution Factor:	A.M.: 958 P.M.: 1,078	N: 271 (28%) N: 672 (62%)	S: 687 (72%) S: 406 (38%)	
Existing Peak Hour Traffic:	A.M.: 958 P.M.: 1,078	(7:00 AM - 8: (4:30 PM - 5:3	00 AM) 30 PM)	
Peak Hour Factor:	0.95			

If you have questions or need additional information, please contact me at (540) 387-5228.

cc:	Alex Vandyke	Joyce Barkley
	Bruce Penner	Jason Henry
	L.J. Muchenje	Anne Booker
	Alex Price	Ray Varney
	Michael Gray	



### I-81 Corridor-wide Design Guide





#### Appendix D: Bridge Widening vs. Replacement

#### Information needs:

IMPROVE 81

- Last bridge inspection report or previous two reports
- Current load rating
- Testing if time allows
  - o Half Cell Potential
  - o Chloride ion profile
  - Depth of cover survey
  - o Visual assessments of top and bottom of deck
  - o Delamination survey

#### **References:**

• VDOT S&B Manual Part 2, Ch 32

#### **Basic Terms:**

- Bridge For VDOT, anything that has a clear span above 20 feet or has a hydraulic opening greater than 36 square feet is considered a *bridge*.
- Condition State General assessment of the quantity of a major bridge component assigned by bridge inspection ranging from 1 (good) to 4 (poor). Related to, but not explicitly tied to, GCR. Typically expressed as a percentage rating for each component.
- Culvert A conveyance for water (or animals) that can be either a box culvert (with or without a bottom), a series of closely spaced pipes (metal or concrete) or an engineered system (i.e., Conspan).
- Deck Roadway surface carrying traffic that is assigned a GCR.
- General Condition Rating (GCR) The controlling rating assigned by the bridge inspection team for a specific bridge element. A total of 3 GCRs are assigned to a bridge and 1 GCR is assigned to a culvert. A bridge with **all** GCRs > 6 is in *Good* condition. A bridge with **any** GCRs < 5 is considered *Structurally Deficient*.
- Hydro-demolition Milling technique utilizing high pressure water to remove concrete deck.
   Shallow = Type A = 1/2" below the milled surface; Deep = Type B = 1" below top mat of rebar across entire deck surface
- Superstructure Supporting elements directly below the deck that is assigned a GCR. Sometimes referred to as beams or girders.
- Substructure Supporting elements directly below the superstructure that is assigned a GCR. Sometimes referred to as piers, bents, abutments, and foundations.

#### IMPROVE 81

- Foundation Supporting elements that are part of the substructure but are typically beneath the ground surface. Typical elements are footings or piles.
- NBIS National Bridge Inspection Standards (NBIS) defines how bridges are inspected. Sometimes, GCRs are referred to NBIS ratings.

#### Process:

- 1. Evaluate the potential bridge repairs
  - o Decks
    - GCRs > 6 and no more than 4% of the deck is in Condition State 2 or greater only minor patching required.
    - GCR <5, deck replacement is required
      - Possible to perform testing and if results favorable, hydro-demolition could be substituted
    - All other GCRs and require hydro-demolition and rigid overlay
      - GCR > 6, Patch and apply flexible overlay
      - GCR = 6, shallow hydro-demolition and rigid overlay required unless
        - Testing is completed and results favorable for standard milling and flexible or rigid overlay
      - GCR = 5, deep hydro-demolition and rigid overlay required *unless* 
        - Testing is completed and results favorable for shallow hydro-demolition and rigid overlay
    - Expansion joints shall be evaluated against existing structure capacities to determine maximum joint closure opportunities using *link slabs*
    - For parapets on non-widening side, only poor condition or bolt-down style need be replaced
    - Bridge drainage shall be considered, and new drains added as needed
    - Wingwall transitions shall be evaluated for non-widened side
  - o Superstructure
    - Evaluate Vertical Clearance including any future widening considerations
    - Strengthening may be required for structures with an existing load positing based upon the current load rating.
    - Repair all damage to beams/girders that are to remain
    - For steel girders, full painting is required unless more than 95% of the existing coating system is in Condition State 1. Otherwise, spot/zone painting applies
  - o Substructure
    - Repair all damage to elements that are to remain
    - Evaluate any scour concerns and address with countermeasures
    - If abutment modification is in scope, consider Virginia micro-abutment in lieu of traditional Virginia Abutment
    - Approach slab may require replacement or extension onto shoulder
  - o Culvert
    - Steel culverts require flow lines unless hydraulic characteristics are not acceptable
    - Concrete culverts require repair as needed
- 2. Evaluate repair cost versus new bridge cost

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- If repairs plus widening are >65% of replacement, additional discussions and justification will be required if replacement is not selected
- Consider all costs including MOT, RW, utilities, environmental, etc.
- 3. Justify and document decision

#### Notes:

• Recommend consultation with District S&B office



### I-81 Corridor-wide Design Guide

Appendix E







#### Appendix E: Stage 1 Bridge Report

#### **Stage I Bridge Report**

For instructions, click <u>here</u>. Facility Carried:

Project Name:		Submittal Date:	
Federal Project	Number:	Fed Structure ID (New):	
State Project Number:		Fed Structure ID (Existing):	
District:	UPC:	Maintenance Authority:	
County:	VA Struc. #:	Funding Source:	
Fed. Oversight	: NFONFONFO	Programmed for Federal Aid:	





#### Prepared for the Virginia Department of Transportation Structure and Bridge

Role	Name	Title	Email	Phone
S&B POC				
Project Manager				

#### Submitted by:

Role	Name	Title	Email	Phone
Designer				
.POC & PM				



#### **Roadway Coordination Data Summary**

Data not shown is provided on roadway plans provided as supplementary data

Based on Roadway Plans dated:

		On Bridge	
Current ADT(year):		Design ADT(year):	
Design Speed:	mph	Posted Speed(mph):	mph
% Trucks:		Profile type	
Reduced Design Speed (if applicable)	mph	Is the road carried on the NHS?	
Functional Classification:		Min. Design Standard:	
Existing Dimensions (If applicable)		Proposed Dimensions	
Max Grade			

#### (if applicable) Under Bridge -

Current ADT(year):		Design ADT(year):	
% Trucks:		Posted Speed(mph)	mph
Design Speed:	mph	Profile type	
Reduced Design Speed	mph	Is the road carried on	
(if applicable)		the NHS?	
Functional		Min. Design	
Classification:		Standard:	
Existing Dimensions		Proposed	
		Dimensions	
Max Grade			



#### **Coordination with Central Office:**

Copy the first two lines of the bridge description on the TS&L cover sheet here.

<b>Project Name:</b> Enter the name of the ProjectEnter the name of the		Submittal Date: Click here for date.Click here for date.Click here		
ProjectEnter the name of the Project		for date.		
Federal Project Number: Enter Federal Project #Enter Federal		Ead Structure ID (New): Eadered ID for Drapaged or N/A		
Project #Enter Federal Project #		red Siluciule ID (New). Federal ID for Proposed of N/A		
State Project Number: Enter State Project #, BXXXEnter State		Ead Structure ID (Existing): Eadered ID for Existing N/A		
Project #, BXXXEnter State Project #, BXXX		Fed Structure ID (Existing). Federal ID for Existing, N/A		
District: Enter DistrictEnter	IIPC: Enter LIPC #	Maintonanco Authority: Choose an item		
DistrictEnter District		Waintenance Authonity. Choose an item.		
County: Enter CountyEnter	VA Struc #: \/A Struc #	Funding Source: Chasse SourceChasse SourceChasse Source		
CountyEnter County	VA Struc.#. VA Struc. #.	Funding Source. Choose SourceChoose SourceChoose Source		
Ed Overeight NEONEONEO		Programmed for Federal Aid: Choose an		
reu. Oversigiit: NFONFONFO		item. Choose an item. Choose an item.		

Work related to the complex items or areas including, but not limited to, Analysis, Design, Detailing, Construction, including Staged Construction, and Special Provisions shall be coordinated with the VDOT Central Office Structure and Bridge Division Complex Bridge and ABC Support Section during Stage II.

#### The following items are designated as complex items or areas in the approved bridge concept:

List complex i	tem/area here			
List complex i	tem/area here			

The following items require a waiver to be completed:	Before Stage I approval	Before PAC	Before Field Inspection (FI)
List item requiring a waiver.			
List item requiring a waiver.			

Note: The waiver shall describe a plan to be followed to ensure the complex items are wholly thought out and will be analyzed with the appropriate level of detail, designed to applicable specifications, are constructible, and will be low maintenance elements in the new bridge.



#### **Approval of Recommended Bridge:**

Copy the first two lines of the bridge description on the TS&L cover sheet here.

Project Name: Enter the name of the ProjectEnter the name of the		Submittal Date: Click here for date.Click here for date.Click here	
ProjectEnter the name of the Project		for date.	
Federal Project Number: Enter Federal Project #Enter Federal		End Structure ID (New): Endered ID for Dropoged or N/A	
Project #Enter Federal Project #		red Structure in (New). rederaring for Proposed of N/A	
State Project Number: Enter State Project #, BXXXEnter State		End Structure ID (Existing): Endered ID for Existing N/A	
Project #, BXXXEnter State Project #, BXXX		red Structure in (Existing). Federal in for Existing, N/A	
District: Enter DistrictEnter	LIPC: Enter LIPC #	Maintonanco Authority: Choose an item	
DistrictEnter District		Mannenance Authority. Gloose an item.	
County: Enter CountyEnter	VA Strue #: VA Strue #	Funding Courses Chasse CourseChasse Course Chasse Course	
CountyEnter County	VA Struc.#. VA Struc. #.	Funding Source: Choose SourceChoose SourceChoose Source	
Fed. Oversight: NFONFONFO		Programmed for Federal Aid: Choose an	
		item Choose an item Choose an item	

Recommend for approval by:	Designer.	Date:
Recommend for approval by:	POC&PM.	Date:
Remarks :		

APPROVAL STATUS:	⊠ Approved
District Structure and Bridge	e Engineer:
Remarks:	
APPROVAL STATUS:	⊠ Approved
Assistant State Structure an	d Bridge Engineer:
Remarks:	
APPROVAL STATUS::	⊠ Approved
State Structure and Bridge E	ngineer:
Remarks:	
APPROVAL STATUS::	⊠ Approved



FHWA Virginia Division Bridge Engineer

Remarks:

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#### 1.0 Superstructure

Design Exceptions, Waivers, or Alternative Technical Concepts are required: 
Yes, see Table 1 below 
No

Table 1.0: Applicable Design Exception/ Waiver/ATC for Superstructure and Superstructure Elements

	Check if violated						
Bridge Element	Tech. Reqs (DB & P3 Only)	S&B Design Manual	I&IM' s	AASHTO	Supporting Documentation*	Approved	Date
Proposed shoulders							
Bike and or Ped. facilities							
Parapet, Rail, and Ped Fence							
Median geometrics							
Soundwalls (size,location, material)							
Sight Distance (Horiz. & Vert.)							
Deck Width Limitation for Longitudinal Joints							
Proposed Transverse Joints							
Hor. And Vert. clearances							
Other:							

Table 1 Superstructure Applicable Design Exception/Waiver/ATC *see related section below for justification and data

* Indicate the name of the Exception/Waiver/ATC and guidance as to where it is stored via hyperlink, description or other.

NOTE: All citations shall include a specific reference:

- 1. Technical Requirement including section number and paragraph
- 2. Structure & Bridge Instructional and Information Memoranda (I&IM's) including the version, year and section.
- 3. Department S&B Design Manual including File Number and page
- 4. Department Road and Bridge Specifications: Section and Subsections
- 5. AASHTO including Document Title & Edition chapter and section
- 6. All other references shall include a detailed citation



#### 1.1 Span Layout

The structure is jointless as described in S&B Design Manual:   Yes	□ No, refer to <u>Table 1</u> and <u>Table 1.4</u>
Expansion Joints	

Table 1.1 Span Layout

lloit #	# Spans	Span	Sk	æw	
Unit #	per Unit	Length(s)	Begin span	End span	onit type

#### 1.2 Superstructure Geometrics

The following VDOT S&B shoulder requirements have been selected based on a functional roadway classification of . For additional geometrics considerations see narrative below.

 Table 1.2.a Superstructure Geometrics

From:	Dimension
Width is Face to Face of	

**Width include widening (of on left of traffic (and) on right of traffic.)

The structure provides lane(s) that

Vertical clearances provided exceed minimum required vertical clearance: 
Yes No, see description below

Table 1.2.b Vertical Clearances

List all spans numerical order	Controlling Feature	Minimum Vertical Clearance Provided (ftin)			
Span 1					

#### 1.3 Railings, Bike & Pedestrian Facilities

Table 1.3 Minimum required railing test level selection

Railing Location		Test Level Req'd	Test Level Prov'd	Proposed Railing Type
Exterior (S&B Design Manual File 25.02)				
Interior(S&B Design Manual File 6.04)	Median:			
	Shared Use			
	Path:			

#### 1.4 Longitudinal and Transverse Joints

#### Table 1.4 Joint Index Table

Transverse Joint Location	Transverse Joint Type	Waiver/ATC	
		Yes, Refer to Table 1	□No
Longitudinal deck joint Choose an	n item. required; deck width Choose an iten	n. to S&B Design Manual 10.01-1	ID.

Refer to Table 1

#### 1.5 Ability to Inspect and Maintain Superstructure

Table 1.5 Access for Inspection and Maintenance

Location	Description	Waiver/ATC
Vertical and Horizontal AlignmentsVertical and Horizontal AlignmentsVertical and Horizontal Alignments	<ul> <li>Req'd. horiz. clr.      IS      IS NOT provided.</li> <li>Req'd. vert.¹ clr.      IS      IS NOT provided.</li> <li>Access to adjacent element      IS      IS NOT restricted for inspection and maintenance by industry standard equipment.</li> <li>TRS      HAVE      HAVE NOT been violated</li> </ul>	<ul> <li>☐ Yes, Refer to <u>Table 1</u></li> <li>☐No</li> </ul>
Maintenance Jacking and Blocking (future)Maintenance Jacking and Blocking (future)Maintenance Jacking and Blocking (future)	<ul> <li>Req'd. horiz. clr.   IS   IS NOT provided.</li> <li>Req'd. vert. clr.   IS   IS NOT provided.</li> <li>Access to adjacent element   IS  IS NOT restricted for inspection and maintenance by industry standard equipment.</li> <li>TRS   HAVE   HAVE NOT been violated</li> </ul>	☐ Yes, Refer to <u>Table 1</u> ☐No
	<ul> <li>Req'd. horiz. clr. □ IS □ IS NOT provided.</li> </ul>	☐ Yes, Refer to <u>Table 1</u> □No



<ul> <li>Req'd. vert. clr. □ IS □ IS NOT provided.</li> </ul>	
<ul> <li>Access to adjacent element</li></ul>	
□IS NOT restricted for inspection and maintenance by industry standard equipment.	
• TRs 🗆 HAVE 🗆 HAVE NOT been	
violated	

#### 1.0 2.0 Substructure

Design Exceptions, Waivers, or Alternative Technical Concepts are required: 
Yes, see Table 2 below 
No

Table 2.0: Applicable Exception/Design Waiver/ATC for Substructure and Substructure elements

		Check if violated					
Bridge Element	Tech. Reqs (DB & P3 Only)	S&B Design Manual	I&IM' s	AASHTO	Supporting Documentation	Approved	Date
Abutment Selection TypeAbutment Selection TypeAbutment Selection Type							
Strength of ConcreteStrength of ConcreteStrength of Concrete							
Other:							

Table 2 Substructure Applicable Design Exception/Waiver/ATC *see related section below for justification and data

NOTE: All citations shall include a specific reference:

- 1. Technical Requirement including section number and paragraph
- 2. Structure & Bridge Instructional and Information Memoranda (I&IM's) including the version, year and section.
- 3. Department S&B Design Manual including File Number and page
- 4. Department Road and Bridge Specifications: Section and Subsections
- 5. AASHTO including Document Title & Edition chapter and section
- 6. All other references shall include a detailed citation

#### 2.1 Abutments

Table 2.1: Abutment Type Selection

Abutment	Туре	Minimum Provided Horizontal Clearance* (ftin)	Abutmer Design N	nt conforms to S&B Manual Chapter 17	Abutment conforms to other requirements (AREMA, etc.)		
Abutment A			□Yes	□No, see discussion below	□Yes	□No, see discussion below	
Abutment B			□Yes	□No, see discussion below	□Yes	□No, see discussion below	

#### 2.2 Piers

Table 2.2: Pier Type Selection

Pier Number	Туре	Minimum Provided Horizontal Clearance* (ftin)	Pier conforms to S&B Design Manual Chapter 15		Pier Designed for Collision	
Pier 1			□Yes	□No, see discussion below	□Yes	□No, see discussion below

#### 2.3 Substructure Protection

#### Table 2.3: Substructure Protection

Proposed Protection for	Туре	Minimum Provided Horizontal Clearance to Bridge Item* (ftin)	Exception/Waiver/ATC	
			□Yes, refer to <u>Table 1</u> □No	

Adjust table as necessary, refer to Road and Bridge plan sheets for more detail

*Minimum gap distance provided between substructure unit and protection.

#### 2.4 Proposed Foundations

#### Table 2.4: Substructure Foundation Selection

Proposed Foundation for

Туре

#### 2.5 Fills and Retaining Walls

New fill(s) impact(s) adjacent facilities: 
Yes, see narrative description below 
No impact is foreseen

New retaining wall(s) impact(s) adjacent facilities: 
Yes, see narrative description below 
No impact is foreseen

#### 2.6 Geotechnical Description

Table 2.6: Substructure Foundation Selection

Existing Foundation for	Туре

Proposed settlement criteria and mitigation:

Geotechnical material description:

Existing structure plans are attached **Yes**, see appendix

□No, existing structure information not available

#### 2.7 Ability to Inspect and Maintain Substructure

Location	Description	Waiver/ATC
Vertical and Horizontal AlignmentsVertical and Horizontal AlignmentsVertical and Horizontal Alignments	<ul> <li>Req'd. horiz. clr.   IS   IS NOT provided.</li> <li>Req'd. vert.¹ clr.   IS   IS NOT provided.</li> <li>Access to adjacent element   IS   IS NOT restricted for inspection and maintenance by industry standard equipment.</li> <li>TRs   HAVE   HAVE   HAVE NOT been violated</li> </ul>	☐ Yes, Refer to <u>Table 1</u> ☐No
	<ul> <li>Req'd. horiz. clr. □ IS □ IS NOT provided.</li> <li>Req'd. vert. clr. □ IS □ IS NOT provided.</li> <li>SAFE access to element □ IS □IS NOT provided for inspection and maintenance by industry standard equipment.</li> <li>Access platform □ HAS □HAS NOT been provided.</li> <li>TRs □ HAVE □ HAVE NOT been violated</li> </ul>	☐ Yes, Refer to <u>Table 1</u> ☐No

#### Table 2.7 Access to Substructure for Inspection and Maintenance

### 2.0 3.0 Site Description

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#### 3.1 Project Location

The project is located in INSERT COUNTY near prominent easily identifiable location.on type of terrain.. The structure carries route # over insert feature intersected. The bridge is on new or existing alignment.

#### 3.2 Adjacent Projects, Structures, Businesses & Residences

The project has impacts to adjacent structures, :  $\Box$ Yes, see narrative description below  $\Box$ No

#### 3.3 Environmental

Environmental Impact(s) to project .Wetlands, flood plains, floodways, tidal | 🗆 Yes, confirmed impact 🗆 No impact, confirmed 🗆 Investigation Underway 🗆 N/A .Historical properties | 
Yes, confirmed impact 
No impact, confirmed 
Investigation Underway 
N/A 4F properties | 🗆 Yes, confirmed impact 🗆 No impact, confirmed 🗆 Investigation Underway 🗆 N/A Agricultural district properties | 🗆 Yes, confirmed impact 🗆 No impact, confirmed 🗆 Investigation Underway 🗆 N/A Northern Long-Eared Bats | 
Yes, confirmed impact 
No impact, confirmed 
Investigation Underway 
N/A Dredging | I Yes, confirmed impact I No impact, confirmed I Investigation Underway I N/A Potential hazardous material sites | Pyes, confirmed impact No impact, confirmed Investigation Underway N/A .Nesting and migrating birds | 🗆 Yes, confirmed impact 🗆 No impact, confirmed 🗆 Investigation Underway 🗆 N/A Migrating fish | 
Yes, confirmed impact 
No impact, confirmed 
Investigation Underway 
N/A Soundwall requirements | Ves, confirmed impact No impact, confirmed Investigation Underway N/A Cofferdams | 
Yes, confirmed impact 
No impact, confirmed 
Investigation Underway 
N/A SWM facilities | D Yes, confirmed impact D No impact, confirmed D Investigation Underway N/A Construction access restrictions | 🗆 Yes, confirmed impact 🗆 No impact, confirmed 🗆 Investigation Underway 🗆 N/A Temp. causeway or work bridge restrictions | Yes, confirmed impact D No impact, confirmed D Investigation Underway D N/A EA/EIS requirements | 🗆 Yes, confirmed impact 🗆 No impact, confirmed 🗆 Investigation Underway 🗆 N/A Time of Year Restrictions | Yes, confirmed impact No impact, confirmed Investigation Underway N/A Other, specified below | D Yes, confirmed impact D No impact, confirmed Investigation Underway N/A

The environmental permit



#### 3.4 Railroad, WMATA, other Rail type 3.4.1 Railroad

Table 3.4: Railroad Considerations

Railroad	Number of existing tracks	Number and location of future track requirements	Shoring required	Crashwall required	Flagging required	Pedestrian Fence required
			□Yes □No	□Yes □No	□Yes □No	□Yes □No

#### 3.4.2 WMATA

Washington Metropolitan Area Transit Authority, future and existing conditions, Choose an item. in the project location. A description of special requirements, clearances, construction constraints and all other that apply to WMATA Choose an item. highlighted below.

#### 3.4.3 Other Rail Type

#### 3.5 Constructability Issues

Sensitive structures, utilities, businesses and/or residences are located within the zone of influence (ZOI) of vibration inducing construction activities:  $\Box$ Yes, see narrative description below.  $\Box$  No mitigation required.

#### 3.7 Coast Guard Issues

#### 3.8 Utilities

Table 3.8: Utility Considerations (Document all utilities which may be impacted by construction at this site.)

Type of Utility	Location of Utility	Company	Description of Utility provided Below	
			□Yes	□No

Insert a narrative description for utilities and proposed mitigation; if necessary to clarify information provided and justify requests for any Design waivers/Exception. If applicable specify authority to perform utility work, payment authority and any other type of agreement that could be related to a utility item.

#### 3.9 Aesthetic Considerations

#### 3.10 Traffic Engineering

#### 3.11 Drainage

Deck drains Choose an item.

#### 3.12 Hydrologic and Hydraulic

Executive Summary from the preliminary H&H report: 
Included in appendix 
In development 
Not 
required/applicable

#### Table 3.12: Hydraulic

H&H Item	H&H Data
FHWA construction/scour code	
Drainage Area in ft ²	
Design Flood elev.	
Historic flood elev. (Date)	
Min. bridge length and skew req'd	

Scour susceptibility and required countermeasures:

#### 3.0 4.0 Future Considerations of Proposed Structure

4.1 Future widening Considerations:

#### 4.2 Future Deck Replacement Considerations:

4.3 Bridge Inspection Considerations

4.4 Bridge Maintenance Considerations

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#### 4.5 Other Considerations

#### 4.0 5.0 Bridge Preliminary Recommendation

- 5.0 6.0 Alternative Bridge Recommendation
- 6.0 7.0 Engineer's Cost Estimate for each Alternative
- 7.0 8.0 Schedule

#### 8.0 9.0 Appendix

9.1 Bridge Plans (to include Sequence of Construction)

9.2 Roadway Plans

9.3 Existing Bridge Plans

9.4 Preliminary Geotechnical Report

9.5Preliminary Hydrologic and Hydraulic Report

9.6 Data Sources

Data from the following sources was considered in the development of this report

Environmental Sources					
□CE	□EA □EIS □Other				
L&D Sources	L&D Sources				
	Roadway Design Plans				
	Traffic Engine	Traffic Engineering Reports			
Surveys					
	Bridge Situation Plan				
	Topographic survey and bathymetric/hydraulic datum references				
	Hydraulics/Hydrology				
	Utility Survey				
	Other Survey (scour, etc.)				
Bridge Sources					
	Existing Bridge Plans				
	Existing Bridge Inspection Report(s)				
	Other				
Geotechnical sources					

	Existing Geotechnical Information (From project site, nearby bridges, roadway borings, geologic maps, etc.)
	Project Specific Geotechnical
Meeting(s) (Loc	alities, District, Residency, etc.) (dates on for these are critical)
	Localities
	District
	Residency
	Other
Other Sources	
	FHWA Recommendation (Rehabilitation vs. Replacement)
	Site Visit
	Other







## I-81 Corridor-wide Design Guide

Appendix F





### Appendix F: Replacing Guardrail Within Project Limits

#### Information needs:

- Existing roadway plans showing typical sections and guardrail placements
- Proposed roadway plans showing typical sections and guardrail placements
- Aerial imagery of roadway
- Survey of pavement edge lines, slope contours and existing lane striping
- Guardrail assessment on existing guardrails where potential to remain exists
- Existing roadway crash data and analysis

#### **References:**

- AASHTO Green Book Chapter 4.4.2 (Shoulder Width)
- VDOT Road Design Manual (RDM) (Appendix J)
- VDOT Road & Bridge Standards
- VDOT I&IM IIM-LD-222/ IIM-TE-358 NCHRP 350 Test Requirements
- VDOT I&IM IIM-TE-366 Barrier Systems

#### Process:

- Conduct constraints analysis identifying unique project constraint locations in context of the entire corridor.
- Follow current VDOT policy outlined in Appendix J of the Road Design Manual which requires replacement of all deficient guardrail within project limits.
- Assess and evaluate guardrail replacement needs
  - Upgrading Existing
    - o Determine what runs of guardrail are directly impacted by construction activities.
      - For example, an inside shoulder run would not be directly impacted by construction if a project is solely an outside widening.
    - Determine existing shoulder width.
    - Determine existing guardrail deficiency through guardrail assessment utilizing VDOT I&IM IIM-TE-366
    - If deficient, determine if more than 60% of the existing substandard run of guardrail is within the project limits as per RDM Appendix J.
      - If limits not exceeded, replace only deficient sections directly impacted by construction. No DE/DW is required for substandard shoulder offsets in areas not directly impacted by construction.
      - If limits exceeded, replace the entire run of guardrail including the end terminals for runs directly impacted by construction. Upgrading a substandard shoulder for areas not directly impacted by construction in runs

being upgraded is not required **except** when required for an end terminal. Additionally, no DE/DW is required for substandard shoulder offsets in areas not directly impacted by construction.

- Upgrades to substandard guardrail in areas not directly impacted by the construction are not required regardless of the 60% threshold. No DE/DW is required for substandard shoulder offsets in areas not directly impacted by construction.
- If a decision is made to upgrade deficient guardrail outside sections directly impacted by construction, consider limiting the scope of the upgrade to the guardrail system and not including shoulder widening that otherwise would not be included within the project scope. No DE/DW is required for substandard shoulder offsets. The overall project budget should be considered when making these decisions.
- Consideration for upgrading/replacing outside the limits of the project and the 60% threshold should be a data driven decision. If prioritization is needed to decide, crash data should be used to support any decision.
- If the data driven process is used, consider what is leading to guardrail hits at a high frequency. Are there pavement issues that could be corrected? Is there a crossslope issue that can be resolved? These alternative solutions may prove to be more cost effective and may provide a safer facility.

#### Length of Need

- Evaluate length of need (LON) for existing guardrail runs.
- Meet minimum standards for LON and consider additional widening costs for closing gaps between adjacent runs of less than 200 feet.
- Utilize long post installations as appropriate to minimize additional shoulder widening.

#### • Shoulder Strengthening

- The I-81 Program has established guidance that paving beneath guardrail as per Road & Bridge Standard MC-4 is generally not required or preferred.
- Under guardrail paving versus asphalt curbing should be evaluated in locations where a high probability of shoulder washout exists.
- Any areas for under guardrail paving should be reviewed with the I-81 Program Manager or his designee.
- Unnecessary shoulder strengthening or paving to meet requirements should be avoided in areas outside the project limits.
- When traffic shifts are utilized, shoulder strengthening should be limited to only areas of need within the shifted lanes.

#### **Other Considerations:**

- Utilize long post guardrail wherever practical and possible to avoid impacting areas that were not planned to be impacted by the project and would only negligibly improve safety.
- Do not provide additional pavement beneath the guardrail outside areas directly impacted by construction to deter grass and weed growth. VDOT has contracts to mow the interstate which includes mechanical methods of removing grass and weeds around guardrail. The extension of pavement under the guardrail is not anticipated to enhance safety or operational characteristics of the interstate system.
- The MGS2 terminals shall be installed with the full site prep as shown on standard 506.07 and not the limited use site prep.

- At the discretion of the VDOT project manager, trench widening or underdrains can be considered to extend to meet new guardrail installations and provide for future use, but only with the following in place:
  - High confidence in the construction cost estimate and schedule.
  - Project has budget available for additional upgrades.
  - Consider including in the contract directly or as a bid additive/alternate.





## I-81 Corridor-wide Design Guide

Appendix G





# Appendix G: Temporary Drainage & Spread at Barrier Wall / Parapet

#### Information needs:

- Existing roadway plans showing typical sections and drainage features
- Proposed roadway plans showing typical sections and drainage features
- Proposed Traffic Management Plan (TMP) typical sections
- Aerial imagery of roadway
- Survey of pavement edge lines and existing lane striping
- Digital terrain model or survey elevations of existing grades
- Existing bridge or structure plans

#### **References:**

- VDOT Drainage Manual (Chapter 9)
- VDOT Manual of the Structure & Bridge Division
- VDOT Road Design Manual (RDM) (Appendix J)
- VDOT Road & Bridge Standards

#### Process:

- Design must consider new requirements for spread in both temporary and permanent conditions as per the VDOT Drainage Manual (Ch. 9.5).
- Evaluate all planned stages of Maintenance of Traffic for constrained conditions and lane widths where spread will encroach into lanes beyond allowable per above.
- Develop solutions to decrease spread.
  - o Potential solutions for consideration may include, but are not limited to the following
    - Add drainage inlets to existing or new bridge decks
      - This may require adding collector pipe under the bridge deck to capture runoff and release to point locations.
    - Modify proposed lane and shoulder widths
    - Adding slot drains to barrier wall / parapet should be considered a last resort and are unlikely to be approved.
- Solutions must be reviewed and approved by the appropriate disciplines (i.e., Drainage, S&B, L&D, etc.)
  - Spread of up to one foot into the travel lane may be approved on existing bridges for temporary maintenance of traffic shifts required for phased construction where other potential mitigations are not feasible.
- Drainage holes may be considered on a case-by-case basis for an existing bridge if the situation is temporary and the bridge will eventually be demolished. A Design Approval shall be requested from the District Bridge Engineer.

- If holes are placed in bridge decks, care must be taken:
  - Avoid top flanges of girders
  - $\circ$  Maximum size  $\leq$  six inches
- If spread requirements cannot be met, a Design Waiver should be pursued through L&D.
- Any areas where overbuilding a structure to address temporary drainage is chosen should be reviewed with the I-81 Program Manager or his designee.

#### Other Considerations

• Temporary drainage should not be the only reason to overbuild a structure. Other considerations including future widening or sight distance mitigation may provide opportunities to address temporary drainage in addition to the other considerations. Overbuilding on bridges should be considered as a last resort.

For phases of MOT beyond the first stage, cross slope correction and grade adjustments should also be considered in reducing potential spread concerns.

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Appendix H





# Appendix H: Design of Access to SWM Facilities in Medians

#### Information needs:

- Existing roadway plans showing typical sections and drainage features
- Proposed roadway plans showing typical sections, drainage features, and SWM facilities
- Aerial imagery of roadway
- Survey of pavement edge lines and existing lane striping
- Digital terrain model or survey elevations of existing grades
- Hydrology & Hydraulics Report

#### **References:**

- VDOT Drainage Manual (Chapter 11)
- VDOT BMP Maintenance Manual
- Virginia Stormwater Management Handbook
- VDOT Road Design Manual (RDM)
- VDOT Road & Bridge Standards

#### Process:

- Design must consider access for routine maintenance and corrective maintenance
  - Typical routine maintenance activities include: inspections, mowing and vegetation management, and litter and debris removal
  - Corrective maintenance activities include: structural repair, partial rehabilitation or rebuild, and significant sediment or debris removal
- Access from travel lanes to SWM facilities within medians to be based on VDOT typical interstate median crossover design, utilizing the following guidance:
  - o VDOT Road and Bridge Standards, CR-1
  - VDOT Road Design Manual Appendix F, Median Crossovers
- Evaluate sight distance for ingress/egress to the median SWM facilities and consider the need for wider shoulder or accel/decel lanes. Accel/decel lanes are not typically appropriate for SWM access but may be considered on a case-by-case basis based on site specific characteristics or safety concerns.
- Consider access requirements, design vehicle for routine maintenance activities, and extent of access to and around the SWM facility
  - Potential items for consideration may include, but are not limited to the following
    - Access point and parking location for vehicle transporting the routine maintenance equipment
    - Unloading and loading of maintenance equipment

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- Turning movements for maintenance equipment traversing the access road around the SWM facility
- "Dump and Trailer" vehicle modified to simulate the characteristics of a single axle dump truck to be modeled in AutoTurn to confirm turning movements (below is a screen show of vehicle and trailer dimensions for typical maintenance design or the I-81 corridor). A different design vehicle may be considered based on site specifics, specific maintenance requirements, or type of proposed BMP facility.
- Evaluate horizontal and vertical alignment of maintenance access road. The Standard PE-1 details shown in VDOT's Road and Bridge Standards should be used for maintenance access road. When practical, grades along the access roads should not exceed 10%.
- Surface material for access road
- Corrective maintenance equipment and access requirements
- Document traffic control requirements during routine maintenance activities
- Consider time of day restrictions for maintenance activities
- Document routine maintenance schedule, inspection, performance requirements, and maintenance of traffic requirements
- Evaluate separation between adjacent lane and limits of SWM facility to confirm clear zone and access requirements are accommodated sufficiently
- Location of SWM facilities and proposed access must be coordinated between the appropriate disciplines (i.e., Drainage, L&D, Interstate Maintenance, Residency, etc.)



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